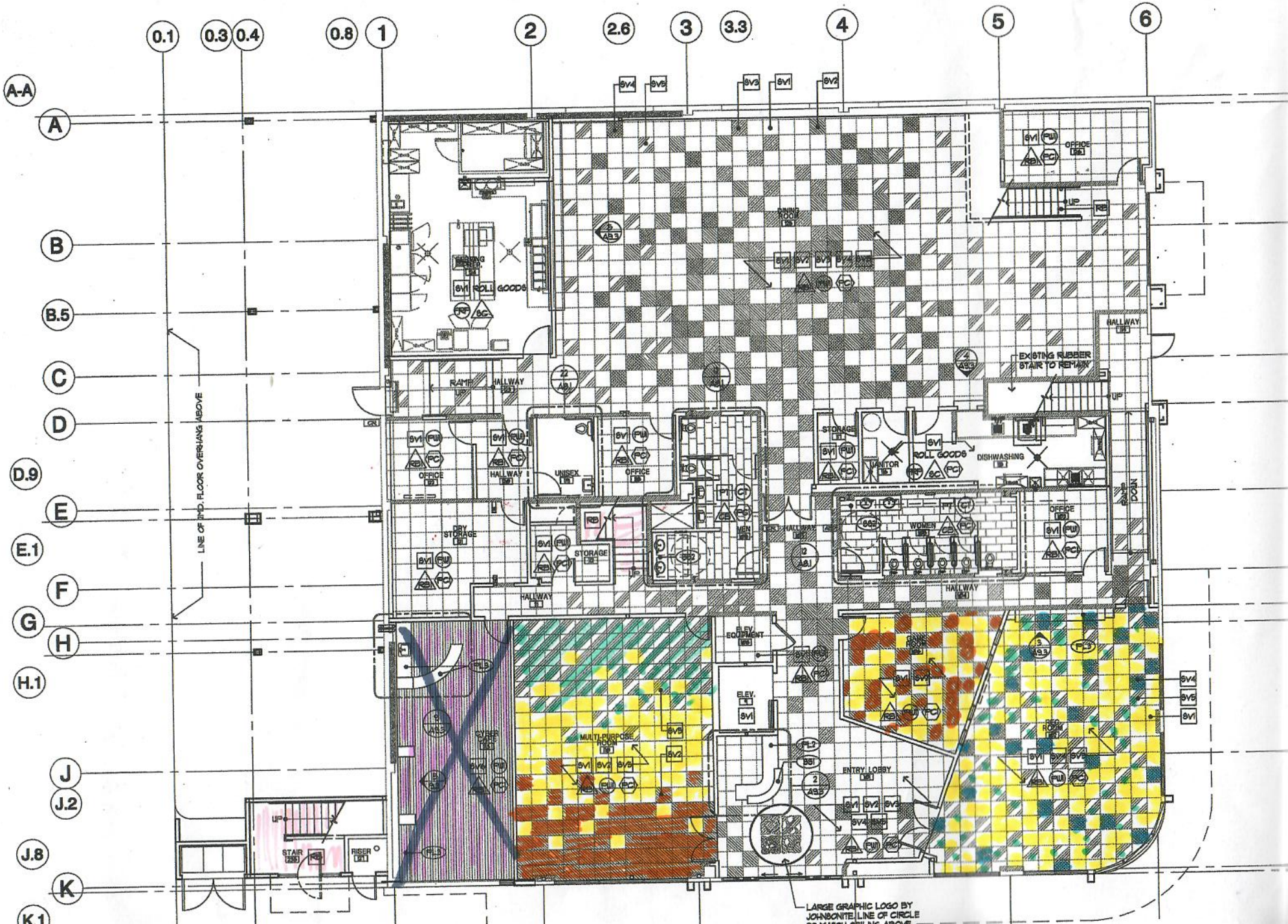
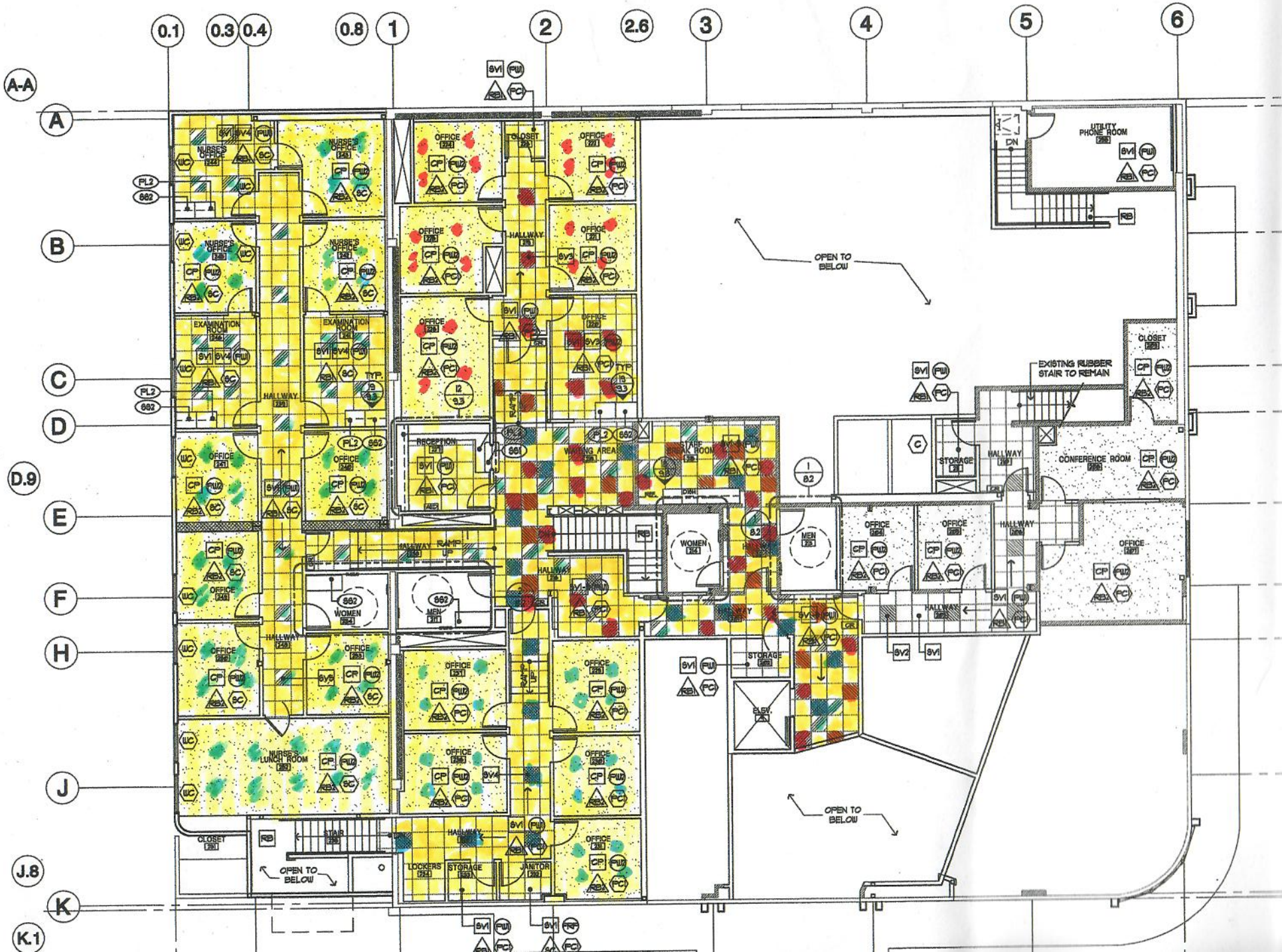
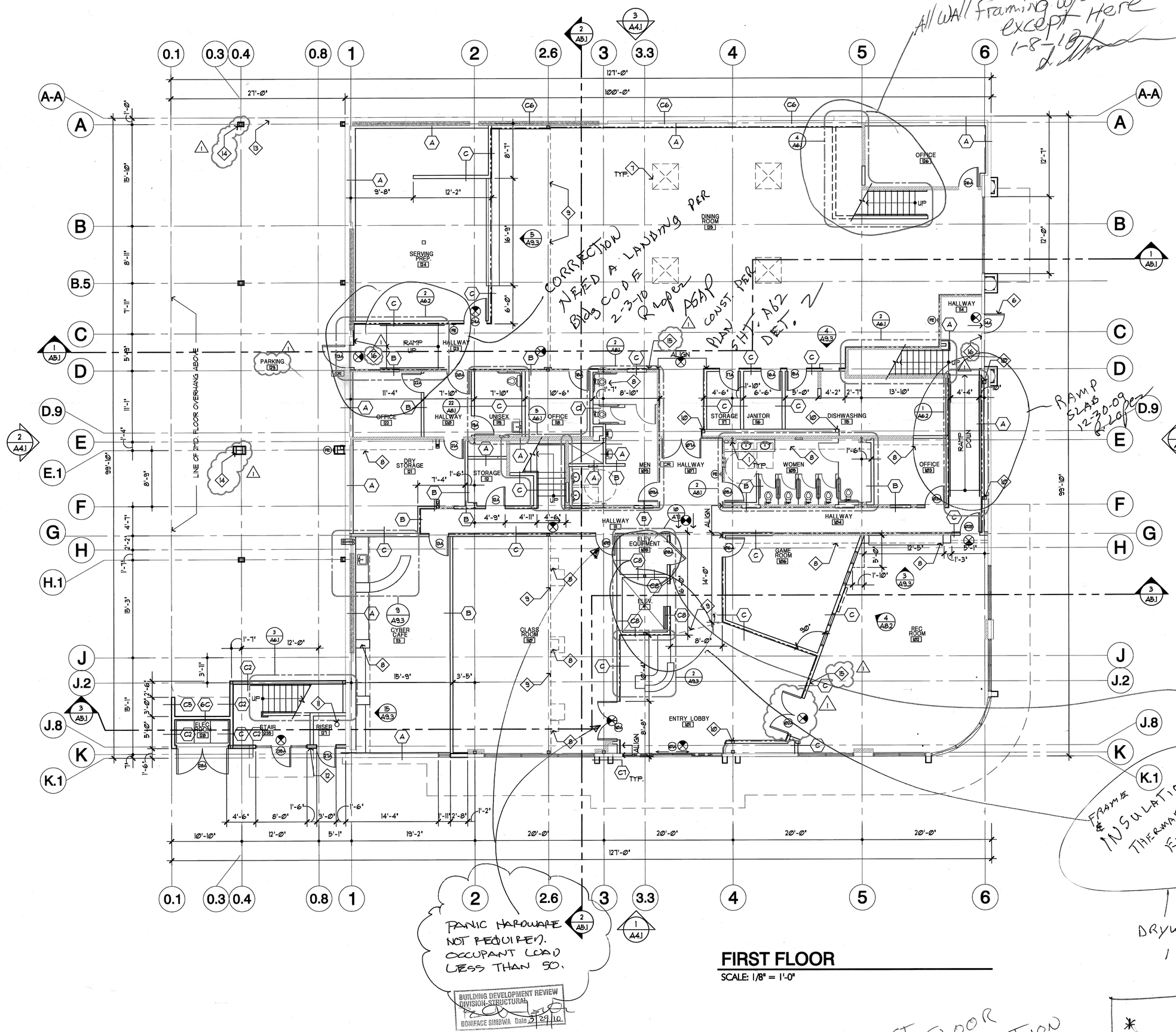

0.1
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A-A
A
B
B.5
C
D
D.9
E
E.1
F
G
H
H.1
J
J.2
J.8
K
K.1
LINE OF 2ND. FLOOR OVERHANG ABOVE
DINING ROOM
ROLL GOODS
RAMP
HALLWAY
OFFICE
UNISEX
STORAGE
DRY STORAGE
MEN
WOMEN
MONITOR
DISHWASHING
EXISTING RUBBER STAIR TO REMAIN
UP
RAIN DOWN
ELEV. EQUIPMENT
ELEV.
MULTI-PURPOSE ROOM
ENTRY LOBBY
REC. ROOM
STAIR
RISER
LARGE GRAPHIC LOGO BY JOHNSONITE, LINE OF CIRCLE


0.1
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A-A
A
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C
D
D.9
E
F
H
J
J.8
K
K.1
OPEN TO BELOW
EXISTING RUBBER STAIR TO REMAIN
NURSE'S OFFICE
EXAMINATION ROOM
OFFICE
HALLWAY
RECEPTION
WAITING AREA
WOMEN
MEN
RAMP UP
RAMP DN
STAIR
ELEV.
STORAGE
CLOSET
LOCKERS
JANITOR
NURSE'S LUNCH ROOM
UTILITY PHONE ROOM
CONFERENCE ROOM
DN
UP
TYP.



FIRST FLOOR

SCALE: 1/8" = 1'-0"

PLAN KEY NOTES:

1. EXISTING STRUCTURAL COLUMN TO REMAIN
2. EXISTING PIPE BOLLARD TO REMAIN
3. ROOF LADDER PER (22 AD3) (23 AD3) (24 AD3)
4. 4'-0" HIGH, 4'X8'X3/4" ACX PLYWOOD TELEPHONE BACKBOARD o/ GYP. BD. WALL PAINTED
5. NEW 4'X8' WHITEBOARD - 36" AFF.
6. EXISTING DOOR TO REMAIN
7. SKYLIGHT ABOVE, SEE ROOF PLAN. (21 AD3)
8. LINE OF SOFFIT ABOVE
9. LINE OF 2ND. FLOOR ABOVE
10. TUBE STEEL BRACE FRAME PER STRUCTURAL DRAWINGS.
11. FIRE SPRINKLER RISER.
12. KNOX BOX @ 6'-0" AFF. VERIFY FINAL LOCATION WITH FIRE DEPARTMENT PRIOR TO INSTALLATION. SEE DETAIL
13. EXISTING CONCRETE RETAINING WALL TO REMAIN
14. NEW STEEL COLUMNS PER STRUCTURAL - SEE (7 AD1) (19 AD1)
15. OCCUPANCY LOAD SIGN - SEE A1.1 FOR ROOM LOAD
16. TACTILE EXIST PER SECTION 1117B.5.1 AND CBC 1011.3 - SEE DETAIL (10 T83)
17. 30"X48" RESCUE ASSISTANCE AREA

SYMBOL LEGEND:

- AED DENOTES NEW CARD READER LOCATIONS.
- CR AUTOMATED EXTERNAL DEFIBRILLATOR - HEIGHT +42"
- PROPOSED DOOR TAG PER A10.1
- DENOTES EXIT SIGN LOCATIONS PER ELECTRICAL DRAWINGS
- FIRE EXTINGUISHER (25 AD1)

NOTE: ALL NOTES MAY NOT APPEAR ON THIS SHEET.

THE PROPOSED AMOUNTS OF HAZARDOUS MATERIALS TO BE USED AND/OR STORED WITHIN THE BUILDING WILL NOT EXCEED THE EXEMPT AMOUNT IN IBC, TABLES 307.1(1) AND/OR 307.1(2).

WALL LEGEND:

- EXISTING INTERIOR WALL: EXISTING INTERIOR 2X WOOD STUD WALLS w/ 5/8" GYP. BD. ON ONE AND OR TWO SIDES TO UNDERSIDE OF FLOOR FRAMING OR UNDERSIDE OF ROOF STRUCTURE.
- EXISTING EXTERIOR WALL: EXISTING EXTERIOR WALLS TO REMAIN UNLESS OTHERWISE NOTED
- EXISTING EXTERIOR WALL: PROVIDE III-IMPACT XP GYP. BD. (BY NATIONAL GYPSUM), TO 48" OF FF. AT HALLWAYS AND HIGH TRAFFIC AREAS.
- (A 3 AD1) FURRING AT EXISTING BLOCK WALL: 2x4 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. ONE SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/R-13 INSUL. IN WALL CAVITY. USE BRICK VENEER CLIPS @ 16" o.c. - SEE STRUCTURAL
- (A1 3 AD1) FURRING AT EXISTING BLOCK WALL: 2x4 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. ONE SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING. USE BRICK VENEER CLIPS @ 16" o.c. - SEE STRUCTURAL
- (B 1 AD1) WALL PARTITION: 2X4 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/R-13 INSUL. IN WALL CAVITY.
- (B1 1 AD1) WALL PARTITION: 2X4 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. EA. SIDE HEIGHT PER PLAN.
- (C 1 AD1) WALL PARTITION: 2X6 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/R-19 INSUL. IN WALL CAVITY.
- (C1 1 AD1) WALL PARTITION: 2X6 WOOD STUDS @ 16" o.c. w/ 5/8" GYP. BD. EA. SIDE HEIGHT PER PLAN.
- (C2 2 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" o.c. TO FLOOR/ROOF STRUCTURE ABOVE, FULL CAVITY w/ R-19 BATT INSULATION, EXTERIOR SURFACE TO HAVE SANDSTONE CEMENT PLASTER WITH MTL. LATH o/ 5/8" DENS GLASS GOLD o/TYVEK o/WOOD STUDS - INTERIOR SIDE TO HAVE 5/8" TYPE 'X' GYP. BD.
- (C3 4 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" o.c. TO ROOF STRUCTURE ABOVE, FULL CAVITY w/R-19 BATT INSULATION, AT EXTERIOR w/(2) ga. GRD GALV. RTL. 1 1/2"X2" PANEL WITH CONCEALED FASTENERS o/TYVEK o/ 5/8" TYPE 'X' DENS GLASS FIREGUARD, AT INTERIOR 5/8" TYPE 'X' GYP. BD. (UL ROTT, FIRE-3-HR, DESIGN U305, U337).
- (C4 1 AD1) WALL PARTITION: 2x6 WOOD STUDS @ 16" o.c. w/ 5/8" GYP. BD. EA. SIDE TO UNDERSIDE OF ROOF FRAMING w/ R-19 INSUL. IN WALL CAVITY.
- (C5 5 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" o.c. TO FLOOR/ROOF STRUCTURE ABOVE FULL CAVITY w/ R-19 BATT INSULATION EXPOSED SIDE TO HAVE SANDSTONE O/ CEMENT PLASTER WITH MTL LATH O/ 5/8" DENS GLASS GOLD O/ TYVEK O/ WOOD STUDS OPPOSITE SIDE TO HAVE A STUCCO FINISH
- (C6 10 AD1) EXTERIOR WALL: 2x6 WOOD STUDS @ 16" o.c. w/ 7/8" PLASTER OVER METAL LATH & WP. PAPER AT EXTERIOR, 5/8" GYP. BD. ON INTERIOR, SEE ELEVATIONS EXTERIOR FINISH.
- (C7 2 AD1) FULL HEIGHT EXTERIOR WALL: (AT POP OUTS) 2x WOOD STUDS EXPOSED SURFACES HAVE SANDSTONE O/ CEMENT PLASTER WITH MTL LATH O/ 5/8" DENS GLASS GOLD O/ TYVEK O/ WOOD STUDS
- (C8 1 AD1) WALL PARTITION: 2X6 WOOD STUDS @ 16" o.c. w/5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/5" OF THERMAFIBER IN WALL CAVITY.

PLAN GENERAL NOTES:

1. INSTALL EXIT SIGNS, PER 2007 CBC SEC 1011, 1013.2
2. REFER TO REFLECTED CEILING PLAN FOR CEILING NOTES, TYPICAL
3. PROVIDE WATER RESISTANT GYP. BD. AT ALL RESTROOM WALLS
4. PROVIDE 1X BLOCKING AS BLOCKING, PLATE FOR ALL UPPER CASEWORK. REFER TO FLOOR PLANS AND INTERIOR ELEVATIONS FOR LOCATIONS
5. SPRINKLER HEADS SHALL ALIGN IN CENTER OF CORRIDOR OR CEILING TILE WHERE OCCURS, USE SEMI-RECESSED SPRINKLER HEADS w/ WHITE ESCUTCHEONS AT SUSPENDED CEILING SYSTEMS AND SEMI-RECESSED SPRINKLERS HEADS w/ FACTORY PRIMED ESCUTCHEONS PAINTED TO MATCH CEILING AT HARDLID AREAS, TYP. SPRINKLER CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR HEAD TYPES AND LOCATIONS TYPICAL.
6. VERIFY EXACT LOCATIONS OF CARD READERS PRIOR TO CONSTRUCTION.
7. CONTRACTOR SHALL KEEP SLAB FREE FROM ALL OIL STAINS, MISC MARKS, ETC, CHALK LINES SHALL BE REMOVED FROM THE SLAB, TYPICAL
8. LATERAL FORCE-BRACING MEMBERS SHALL BE A MINIMUM OF 6 INCHES FROM ALL HORIZONTAL PIPING OR DUCT WORK NOT PROVIDED /w BRACING RESTRAINTS FOR HORIZONTAL FORCES. BRACING WIRES SHALL BE ATTACHED TO THE GRID AND STRUCTURE AND SHALL SUPPORT A DESIGN LOAD THE GREATER OF 100 POUNDS OR THE ACTUAL LOAD, /w A SAFETY FACTOR OF 2
9. ALL WALLS TO RECEIVE 2 COATS OF PAINT AND SHALL BE OF SMOOTH FINISH, AS APPROVED BY ARCHITECT-LEVEL 4 FINISH PER AWCI, GYPSUM ASSOC. ETC.
10. ALL DIMENSIONS SHOWN TO THE FACE OF BLOCK WALL, @ OF COLUMN, OR FACE OF STUD UNLESS OTHERWISE NOTED.
11. ALL GLAZING WILL BE INSTALLED WITH A CERTIFYING LABEL ATTACHED SHOWING THE U-VALUE.
12. LOCATION OF KNOX BOX TO BE APPROVED BY THE FIRE INSPECTOR.
13. PLANS AND ALL FIRE ALARM SYSTEMS SHALL BE SUBMITTED BY THE INSTALLING CONTRACTOR. SUCH PLANS SHALL BE APPROVED BY THE FIRE DEPARTMENT PRIOR TO INSTALLATION.
14. BUILDING SHALL NOT BE ENCLOSED UNTIL FIRE DEPARTMENT APPROVES FIRE SPRINKLER HYDROSTATIC TEST.
15. THIS BUILDING WILL NOT STORE OR USE ANY HAZARDOUS MATERIALS IN EXCESS OF THE QUANTITIES LISTED IN THE CBC TABLES 3-D AND 3-E
16. CONTRACTOR TO VERIFY CONDITION OF EXISTING WOOD AND STEEL MEMBERS AND CONTACT ARCHITECT IF NON-STANDARD CONDITIONS EXIST.
17. CONTRACTOR TO COORDINATE INTERFERENCE WITH MECHANICAL DUCT, ELECTRICAL LIGHTING, ELECTRICAL CONDUIT AND STRUCTURE IN RELATION TO CEILING HEIGHT BEFORE COMMENCING WORK. CONTACT ARCHITECT WITH AREAS OF CONCERN.
18. CONTRACTOR TO FIELD REVIEW FOR WALL INTERFERENCE WITH EXISTING CONDUITS AND PIPING. CONTACT ARCHITECT WITH AREAS OF CONCERN.
19. ALL OVERHEAD SPRINKLERS, UTILITIES AND HVAC DISTRIBUTION TO BE HELD AS TIGHT TO ROOF AS POSSIBLE WITHOUT DIFFICULT OR UNSIGHTLY BENDS. CONTACT ARCHITECT WITH ANY QUESTIONABLE LAYOUTS.
20. CONTRACTOR SHALL NOTIFY ARCHITECT OF INITIAL WALL PARTITION LAYOUT PRIOR TO COMPLETION OF WALL FRAMING FOR REVIEW AND COORDINATION OF WALLS. LOCATION OF WALL PARTITIONS ERECTED WITHOUT THE ARCHITECT'S REVIEW AND APPROVAL SHALL BE NOTED AND IF DEEMED INCORRECTLY LOCATED, CONTRACTOR WILL REMOVE AND/OR RELOCATE AT CONTRACTOR'S OWN EXPENSE AND NO EXPENSE TO THE OWNER.
21. STUD FRAMER TO REVIEW PROPOSED WALL LOCATIONS BEFORE COMMENCING WORK AND CONTACT ARCHITECT IF THERE ARE ANY STRUCTURAL SUPPORT ELEMENTS, UTILITIES, ETC. THAT WOULD PRECLUDE A CONTINUOUS FLUSH FINISH ON GYP. BD. FOR ENTIRE LENGHT OF WALL.
22. PROVIDE WATER RESISTANT GYP. BD. AT MOISTURE SENSITIVE AREAS WHERE GYP. BD. IS SPECIFIED AS A WALL MATERIAL.
23. OFFSET STUDS AS REQUIRED TO CREATE FLUSH FINISH ON ADJACENT WALL SURFACES.
24. CONTRACTOR TO VERIFY THAT ALL TOILET ROOM WALLS AND FLOORS ARE SQUARE, STRAIGHT AND TRUE BEFORE COMMENCING FINISH WORK. UNEVEN TILES AND NON UNIFORM JOINT LINES WILL NOT BE ACCEPTED. LAY OUT TILE TO MINIMIZE CUTS.
25. THOROUGHLY VERIFY ALL FLOOR MATERIAL THICKNESSES TO CONFIRM APPROPRIATE LEVELING / FLOATING. CONTACT ARCHITECT TO OBSERVE AND CONFIRM APPROPRIATE TRANSITIONS. TRANSITIONS SHALL NOT BE VISIBLE TO EYE OR NOTICEABLE WHEN WALKING OVER AREA.
26. TILES TO BE SAWCUT TO ELIMINATE EXTRA JOINTS IN TOILETS ROOMS AND ALL WALLS TO BE SQUARE BEFORE STARTING TILE.
27. CONTRACTOR TO CONTACT ARCHITECT IN ADVANCE IF PATTERN MATCH OR SEAMING ON CARPET WILL BE VISIBLE OTHERWISE CARPET INSTALLATION WILL BE ASSUMED TO BE PERFECT PATTERN MATCH WITH NON VISIBLE SEAMS.
28. ALL FULL HEIGHT INTERIOR NON-LOAD BEARING PARTITIONS MUST HAVE A 1/4" GAP (AFTER ROOF LOADS ARE ADDED) FOR DEFLECTION BETWEEN TOP OF WALL AND STRUCTURE ABOVE.
29. NO PIPES, DUCTS, SLEEVES, CHASES, ETC., SHALL BE PLACED IN SLABS, BEAMS OR WALLS UNLESS SPECIFICALLY SHOWN OR NOTED. CONTRACTOR TO OBTAIN APPROVAL PRIOR TO INSTALLATION OF ANY ADDITIONAL PIPES, DUCTS, ETC.
30. THE PLACEMENT OF PIPES, CONDUITS, OTHER MATERIALS, AND THE LOCATION, SIZE AND REINFORCEMENT OF HOLES IN THE BUILDING STRUCTURE SHALL CONFORM TO THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. WHEN THE REQUIREMENTS OF THE MECHANICAL, ELECTRICAL OR OTHER SECTIONS OF THE SPECIFICATIONS OR DRAWINGS ARE IN CONFLICT WITH THE STRUCTURAL REQUIREMENTS, THE STRUCTURAL REQUIREMENTS SHALL TAKE PRECEDENCE. WHERE THE SAFETY OF THE BUILDING STRUCTURE IS THREATENED, DUE TO MECHANICAL, ELECTRICAL OR OTHER CONSTRUCTION OR HOLES REQUIRED FOR SUCH CONSTRUCTION, MODIFICATIONS SHALL BE MADE AS DIRECTED BY THE ARCHITECT.
31. REFER TO DOOR SCHEDULE FOR ALL DOOR TYPES
32. ALL EXIT DOORS SHALL BE OPENABLE FROM THE INSIDE WITHOUT THE USE OF A KEY OR ANY SPECIAL KNOWLEDGE.
33. PROVIDE A 2A10BC FIRE EXTINGUISHER IN CABINET (SEMI RECESSED @ STUD WALLS, SURFACE MTD. @ BLOCK WALLS) PER CBC SECTION 906. EXACT LOCATION OF FIRE EXTINGUISHERS TO BE DETERMINED BY THE FIRE DEPT. FOR CABINET SEE DETAIL (25 AD1)

1 FLOOR PLANS

SCALE: 1/8" = 1'-0"

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LICENSED ARCHITECT — GERALD S. SMITH — NO. C11701 — REN 11/30/09 — STATE OF CALIFORNIA — cds

www.sca-sd.com

Gary and Mary West Senior Wellness Center

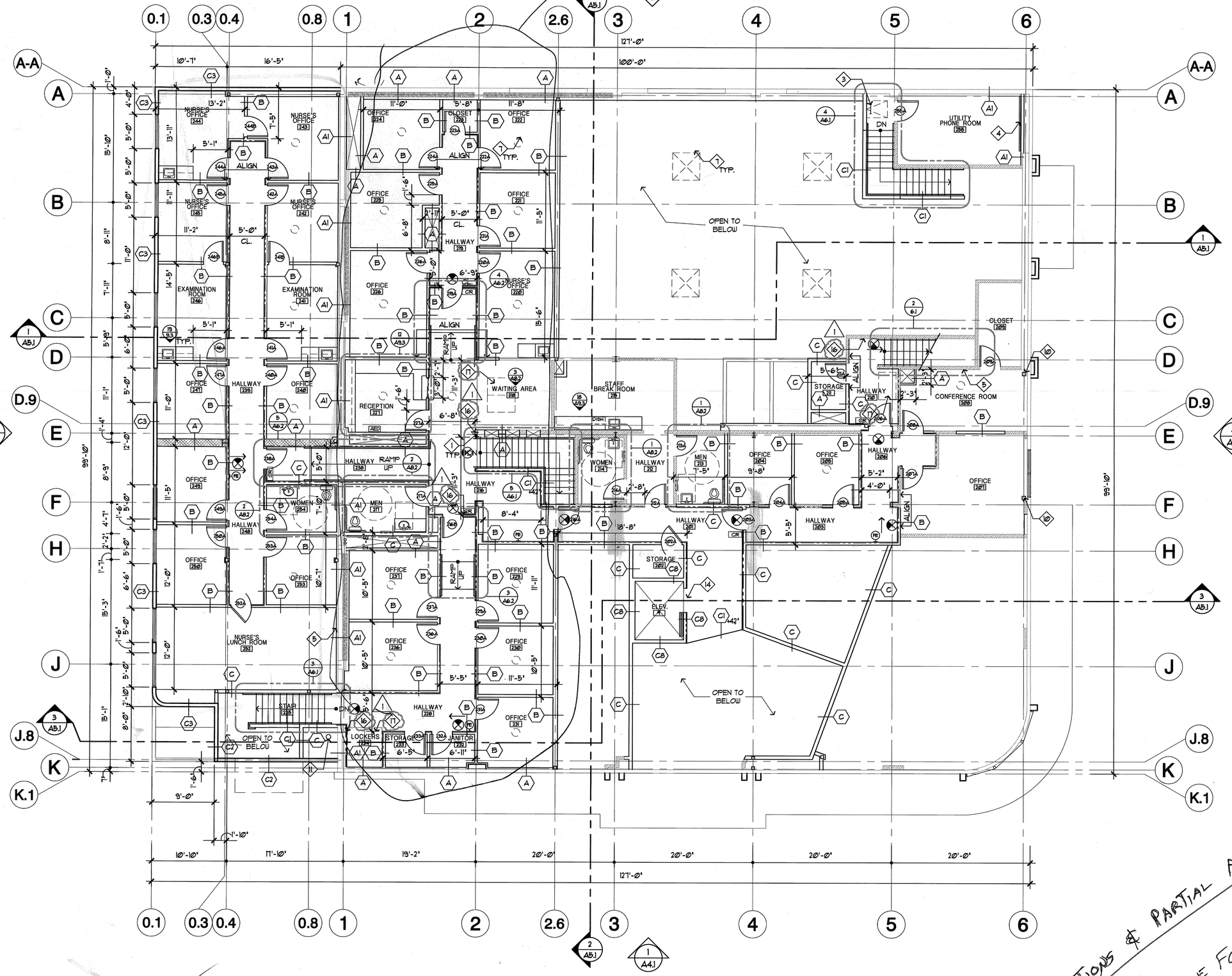
1525 Fourth Avenue
Senior Community Center

Senior Community Centers

Revision Dates	
1	7.16.09
2	.
3	.
4	.
5	.

Issue Dates	
DPR	.
Design Development	.
Plan Check	6.15.09
Bid Set	.
Permit Set	.
Construction Set	.
Drawing Date	.
Check By	SC
Drawn By	jram
Scale	1/8" = 1'-0"
Job Number	06138.T
Sheet Number	A1.1

2ND SUBMITTAL



Rough Frame
INT. WALL
APPROVED PENDING
FLOOR FRAMING
CORRECTIONS
12-24-09
B. Lopez

SECOND FLOOR
SCALE: 1/8" = 1'-0"

From 2/5/10

PLAN KEY NOTES:

1. EXISTING STRUCTURAL COLUMN TO REMAIN
2. EXISTING PIPE BOLLARD TO REMAIN
3. ROOF LADDER PER (22 AD3) (23 AD3) (24 AD3)
4. 4'-0" HIGH, 4'X8'X3/4" ACX PLYWOOD TELEPHONE BACKBOARD o/ GYP. BD. WALL PAINTED
5. NEW 4'X8' WHITEBOARD - 36" A.F.F.
6. EXISTING DOOR TO REMAIN
7. SKYLIGHT ABOVE, SEE ROOF PLAN. (21 AD3)
8. LINE OF SOFFIT ABOVE
9. LINE OF 2ND. FLOOR ABOVE
10. TUBE STEEL BRACE FRAME PER STRUCTURAL DRAWINGS.
11. FIRE SPRINKLER RISER.
12. KNOX BOX @ 60" A.F.F. VERIFY FINAL LOCATION WITH FIRE DEPARTMENT PRIOR TO INSTALLATION. SEE DETAIL
13. EXISTING CONCRETE RETAINING WALL TO REMAIN
14. NEW STEEL COLUMNS PER STRUCTURAL - SEE (7 AD1) (19 AD1)
15. OCCUPANCY LOAD SIGN - SEE AXI FOR ROOM LOAD
16. TACTILE EXIST PER SECTION 1117B.5 J AND CBC 1011.3 - SEE DETAIL (10 TB3)
17. 30"X48" RESCUE ASSISTANCE AREA.

SYMBOL LEGEND:

- AED DENOTES NEW CARD READER LOCATIONS.
- CR AUTOMATED EXTERNAL DEFIBRILLATOR - HEIGHT +42"
- PROPOSED DOOR TAG PER A101
- DENOTES EXIT SIGN LOCATIONS PER ELECTRICAL DRAWINGS
- FE FIRE EXTINGUISHER (15 AD1)

NOTE: ALL NOTES MAY NOT APPEAR ON THIS SHEET.

THE PROPOSED AMOUNTS OF HAZARDOUS MATERIALS TO BE USED AND/OR STORED WITHIN THE BUILDING WILL NOT EXCEED THE EXEMPT AMOUNT IN IBC, TABLES 307.1(1) AND/OR 307.1(2)

2ND FLOOR
MEZZ. DRYWALL
AND INSULATION
APPROVED.
1-11-10 R.Lopez

WALL LEGEND:

- EXISTING INTERIOR WALL: EXISTING INTERIOR 2x WOOD STUD WALLS w/ 5/8" GYP. BD. ON ONE AND OR TWO SIDES TO UNDERSIDE OF FLOOR FRAMING OR UNDERSIDE OF ROOF STRUCTURE.
- EXISTING EXTERIOR WALL: EXISTING EXTERIOR WALLS TO REMAIN UNLESS OTHERWISE NOTED
- EXISTING EXTERIOR WALL: PROVIDE HI-IMPACT XP GYP. BD. (BY NATIONAL GYPSUM), TO 48" OF FF. AT HALLWAYS AND HIGH TRAFFIC AREAS.
- (A) (3 AD1) FURRING AT EXISTING BLOCK WALL: 2x4 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. ONE SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/ R-13 INSUL. IN WALL CAVITY. USE BRICK VENEER CLIPS @ 16" O.C. - SEE STRUCTURAL
- (A1) (3 AD1) FURRING AT EXISTING BLOCK WALL: 2x4 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. ONE SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING. USE BRICK VENEER CLIPS @ 16" O.C. - SEE STRUCTURAL
- (B) (1 AD1) WALL PARTITION: 2X4 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/ R-13 INSUL. IN WALL CAVITY.
- (B1) (1 AD1) WALL PARTITION: 2X4 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. EA. SIDE HEIGHT PER PLAN.
- (C) (1 AD1) WALL PARTITION: 2x6 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/ R-19 INSUL. IN WALL CAVITY.
- (C1) (1 AD1) WALL PARTITION: 2x6 WOOD STUDS @ 16" O.C. w/ 5/8" GYP. BD. EA. SIDE HEIGHT PER PLAN.
- (C2) (2 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" O.C. TO FLOOR/ROOF STRUCTURE ABOVE, FULL CAVITY w/ R-19 BATT INSULATION, EXTERIOR SURFACE TO HAVE SANDSTONE o/CEMENT PLASTER WITH MTL LATH o/ 5/8" DENS GLASS GOLD o/TYVEK o/WOOD STUDS - INTERIOR SIDE TO HAVE 5/8" TYPE 'X' GYP. BD.
- (C3) (4 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" O.C. TO ROOF STRUCTURE ABOVE, FULL CAVITY w/R-19 BATT INSULATION, AT EXTERIOR w/72 ga. GRIP GALV. STL. I 1/2"X2" PANEL WITH CONCEALED FASTENERS o/TYVEK, o/ 5/8" TYPE 'X' DENS GLASS FIREGUARD AT INTERIOR 5/8" TYPE 'X' GYP. BD. (UL R2711, BRK3418, DESIGN U369, U331)
- (C4) (1 AD1) WALL PARTITION: 2x8 WOOD STUDS @ 16" O.C. w/ 5/8" GYP. BD. EA. SIDE TO UNDERSIDE OF ROOF FRAMING w/ R-19 INSUL. IN WALL CAVITY.
- (C5) (5 AD1) FULL HEIGHT EXTERIOR WALL: 2x6 WOOD STUDS @ 16" O.C. TO FLOOR/ROOF STRUCTURE ABOVE, FULL CAVITY w/ R-19 BATT INSULATION EXPOSED SIDE TO HAVE SANDSTONE o/ CEMENT PLASTER WITH MTL LATH O/ 5/8" DENS GLASS GOLD O/ TYVEK O/ WOOD STUDS OPPOSITE SIDE TO HAVE A STUCCO FINISH
- (C6) (10 AD1) EXTERIOR WALL: 2x6 WOOD STUDS @ 16" O.C. w/ 7/8" PLASTER OVER METAL LATH 4 WP. PAPER AT EXTERIOR 5/8" GYP. BD. ON INTERIOR. SEE ELEVATIONS EXTERIOR FOR FINISH.
- (C7) (2 AD1) FULL HEIGHT EXTERIOR WALL: (AT POP OUTS) 2x WOOD STUDS EXPOSED SURFACES HAVE SANDSTONE O/ CEMENT PLASTER WITH MTL LATH O/ 5/8" DENS GLASS GOLD O/ TYVEK O/ WOOD STUDS
- (C8) (1 AD1) WALL PARTITION: 2x6 WOOD STUDS @ 16" O.C. w/ 5/8" TYPE 'X' GYP. BD. EA. SIDE TO UNDERSIDE OF FLOOR OR ROOF FRAMING w/ 5" OF THERMAFIBER IN WALL CAVITY.

PLAN GENERAL NOTES:

1. INSTALL EXIT SIGNS, PER 2007 CBC SEC 1011, 1011.5.2
2. REFER TO REFLECTED CEILING PLAN FOR CEILING NOTES, TYPICAL
3. PROVIDE WATER RESISTANT GYP. BD. AT ALL RESTROOM WALLS
4. PROVIDE 2X BLOCKING AS BACKING PLATE FOR ALL UPPER CASEWORK REFER TO FLOOR PLANS AND INTERIOR ELEVATIONS FOR LOCATIONS
5. SPRINKLER HEADS SHALL ALIGN IN CENTER OF CORRIDOR OR CEILING TILE WHERE OCCURS, USE SEMI-RECESSED SPRINKLER HEADS w/ WHITE ESCUTCHEONS AT SUSPENDED CEILING SYSTEMS AND SEMI-RECESSED SPRINKLERS HEADS w/ FACTORY PRIMED ESCUTCHEONS PAINTED TO MATCH CEILING AT HARD LID AREAS, TYP. SPRINKLER CONTRACTOR TO PROVIDE SHOP DRAWINGS FOR HEAD TYPES AND LOCATIONS TYPICAL.
6. VERIFY EXACT LOCATIONS OF CARD READERS PRIOR TO CONSTRUCTION.
7. CONTRACTOR SHALL KEEP SLAB FREE FROM ALL OIL STAINS, MISC MARKS, ETC, CHALK LINES SHALL BE REMOVED FROM THE SLAB, TYPICAL
8. LATERAL FORCE-BRACING MEMBERS SHALL BE A MINIMUM OF 6 INCHES FROM ALL HORIZONTAL PIPING OR DUCT WORK NOT PROVIDED /s BRACING RESTRAINTS FOR HORIZONTAL FORCES. BRACING WIRES SHALL BE ATTACHED TO THE GRID AND STRUCTURE AND SHALL SUPPORT A DESIGN LOAD THE GREATER OF 100 POUNDS OR THE ACTUAL LOAD, /s A SAFETY FACTOR OF 2
9. ALL WALLS TO RECEIVE 2 COATS OF PAINT AND SHALL BE OF SMOOTH FINISH, AS APPROVED BY ARCHITECT- LEVEL 4 FINISH PER AWCI, GYPSUM ASSOC. ETC.
10. ALL DIMENSIONS SHOWN TO THE FACE OF BLOCK WALL, & OF COLUMN, OR FACE OF STUD UNLESS OTHERWISE NOTED.
11. ALL GLAZING WILL BE INSTALLED WITH A CERTIFYING LABEL ATTACHED SHOWING THE U-VALUE.
12. LOCATION OF KNOX BOX TO BE APPROVED BY THE FIRE INSPECTOR.
13. PLANS AND ALL FIRE ALARM SYSTEMS SHALL BE SUBMITTED BY THE INSTALLING CONTRACTOR. SUCH PLANS SHALL BE APPROVED BY THE FIRE DEPARTMENT PRIOR TO INSTALLATION.
14. BUILDING SHALL NOT BE ENCLOSED UNTIL FIRE DEPARTMENT APPROVES FIRE SPRINKLER HYDROSTATIC TEST.
15. THIS BUILDING WILL NOT STORE OR USE ANY HAZARDOUS MATERIALS IN EXCESS OF THE QUANTITIES LISTED IN THE CBC TABLES 3-D AND 3-E
16. CONTRACTOR TO VERIFY CONDITION OF EXISTING WOOD AND STEEL MEMBERS AND CONTACT ARCHITECT IF NON-STANDARD CONDITIONS EXIST.
17. CONTRACTOR TO COORDINATE INTERFERENCE WITH MECHANICAL DUCT, ELECTRICAL LIGHTING, ELECTRICAL CONDUIT AND STRUCTURE IN RELATION TO CEILING HEIGHT BEFORE COMMENCING WORK. CONTACT ARCHITECT WITH AREAS OF CONCERN.
18. CONTRACTOR TO FIELD REVIEW FOR WALL INTERFERENCE WITH EXISTING CONDUITS AND PIPING. CONTACT ARCHITECT WITH AREAS OF CONCERN.
19. ALL OVERHEAD SPRINKLERS, UTILITIES AND HVAC DISTRIBUTION TO BE HELD AS TIGHT TO ROOF AS POSSIBLE WITHOUT DIFFICULT OR UNSIGHTLY BENDS. CONTACT ARCHITECT WITH ANY QUESTIONABLE LAYOUTS.
20. CONTRACTOR SHALL NOTIFY ARCHITECT OF INITIAL WALL PARTITION LAYOUT PRIOR TO COMPLETION OF WALL FRAMING FOR REVIEW AND COORDINATION OF WALLS. LOCATION OF WALL PARTITIONS ERECTED WITHOUT THE ARCHITECT'S REVIEW AND APPROVAL SHALL BE NOTED AND IF DEEMED INCORRECTLY LOCATED, CONTRACTOR WILL REMOVE AND/OR RELOCATE AT CONTRACTOR'S OWN EXPENSE AND NO EXPENSE TO THE OWNER.
21. STUD FRAMER TO REVIEW PROPOSED WALL LOCATIONS BEFORE COMMENCING WORK AND CONTACT ARCHITECT IF THERE ARE ANY STRUCTURAL SUPPORT ELEMENTS, UTILITIES, ETC. THAT WOULD PRECLUDE A CONTINUOUS FLUSH FINISH ON GYP. BD. FOR ENTIRE LENGHT OF WALL.
22. PROVIDE WATER RESISTANT GYP. BD. AT MOISTURE SENSITIVE AREAS WHERE GYP. BD. IS SPECIFIED AS A WALL MATERIAL.
23. OFFSET STUDS AS REQUIRED TO CREATE FLUSH FINISH ON ADJACENT WALL SURFACES.
24. CONTRACTOR TO VERIFY THAT ALL TOILET ROOM WALLS AND FLOORS ARE SQUARE, STRAIGHT AND TRUE BEFORE COMMENCING FINISH WORK. UNEVEN TILES AND NON UNIFORM JOINT LINES WILL NOT BE ACCEPTED. LAY OUT TILE TO MINIMIZE CUTS.
25. THOROUGHLY VERIFY ALL FLOOR MATERIAL THICKNESSES TO CONFIRM APPROPRIATE LEVELING / FLOATING. CONTACT ARCHITECT TO OBSERVE AND CONFIRM APPROPRIATE TRANSITIONS. TRANSITIONS SHALL NOT BE VISIBLE TO EYE OR NOTICEABLE WHEN WALKING OVER AREA.
26. TILES TO BE SAWCUT TO ELIMINATE EXTRA JOINTS IN TOILETS ROOMS AND ALL WALLS TO BE SQUARE BEFORE STARTING TILE.
27. CONTRACTOR TO CONTACT ARCHITECT IN ADVANCE IF PATTERN MATCH OR SEAMING ON CARPET WILL BE VISIBLE OTHERWISE CARPET INSTALLATION WILL BE ASSUMED TO BE PERFECT PATTERN MATCH WITH NON VISIBLE SEAMS.
28. ALL FULL HEIGHT INTERIOR NON-LOAD BEARING PARTITIONS MUST HAVE A ½" GAP (AFTER ROOF LOADS ARE ADDED) FOR DEFLECTION BETWEEN TOP OF WALL AND STRUCTURE ABOVE.
29. NO PIPES, DUCTS, SLEEVES, CHASES, ETC., SHALL BE PLACED IN SLABS, BEAMS OR WALLS UNLESS SPECIFICALLY SHOWN OR NOTED. CONTRACTOR TO OBTAIN APPROVAL PRIOR TO INSTALLATION OF ANY ADDITIONAL PIPES, DUCTS, ETC.
30. THE PLACEMENT OF PIPES, CONDUITS, OTHER MATERIALS, AND THE LOCATION, SIZE AND REINFORCEMENT OF HOLES IN THE BUILDING STRUCTURE SHALL CONFORM TO THE STRUCTURAL DRAWINGS AND SPECIFICATIONS. WHEN THE REQUIREMENTS OF THE MECHANICAL, ELECTRICAL OR OTHER SECTIONS OF THE SPECIFICATIONS OR DRAWINGS ARE IN CONFLICT WITH THE STRUCTURAL REQUIREMENTS, THE STRUCTURAL REQUIREMENTS SHALL TAKE PRECEDENCE. WHERE THE SAFETY OF THE BUILDING STRUCTURE IS THREATENED, DUE TO MECHANICAL, ELECTRICAL OR OTHER CONSTRUCTION OR HOLES REQUIRED FOR SUCH CONSTRUCTION, MODIFICATIONS SHALL BE MADE AS DIRECTED BY THE ARCHITECT.
31. REFER TO DOOR SCHEDULE FOR ALL DOOR TYPES
32. ALL EXIT DOORS SHALL BE OPENABLE FROM THE INSIDE WITHOUT THE USE OF A KEY OR ANY SPECIAL KNOWLEDGE.
33. PROVIDE A 2A10BC FIRE EXTINGUISHER IN CABINET (SEMI RECESSED @ STUD WALLS, SURFACE MTD. @ BLOCK WALLS) PER CBC SECTION 906. EXACT LOCATION OF FIRE EXTINGUISHERS TO BE DETERMINED BY THE FIRE DEPT. FOR CABINET SEE DETAIL (25 AD1)

1 CORRECTIONS & PARTIAL APPROVAL
2ND FLOOR FRAMING
IS APPROVED Except THE Following, (1) & (2)
(1) LOW STEEL BEAMS
column LINE 3 & 4 MEZZ 2ND FLR.
6'5" / MAINTAIN REQUIRED
CEILING HT. PER Bldg. CODE.
(2) 6X12 WOOD BEAMS NO HANGERS
5DS LAG BOLT 24" OC
(3) Need ALL CITY PLAN CHECK APPROVED CORRECTIONS FOR ABOVE ITEMS & Framing
12-31-09 R.Lopez
PENDING ALL TRADES
* MEZZ.
* FLOOR TRUSSES
FLOOR JOIST
FLOOR BEAMS
NOT PER PLAN
INSP. REQUIRE
WITH NEW
CORRECTIONS

FLOOR PLAN
SCALE: 1/8" = 1'-0"

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Revision Dates	
1	7.16.09

Issue Dates	
DPR	
Design Development	
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Bid Set	
Permit Set	
Construction Set	
Drawing Date	
Check By	SC
Drawn By	Jram
Scale	1/8" = 1'-0"
Job Number	06138.T
Sheet Number	A1.2

2ND SUBMITTAL

SHOTCRETE NOTES

PART 1 - GENERAL

1.1 SUMMARY: THIS SECTION COVERS THE FURNISHING AND PNEUMATICALLY APPLYING A LOW SLUMP CONCRETE AT HIGH VELOCITY ONTO THE PREPARED SURFACE.

1.2 CODES: EXCEPT AS MODIFIED BY THE REQUIREMENTS SPECIFIED HEREIN AND/OR THE DETAILS ON THE DRAWINGS, CONCRETE WORK SHALL CONFORM TO THE "INTERNATIONAL BUILDING CODE" (IBC), 2006, SECTION 1913

1.3 REFERENCES: THE EDITIONS OF THE SPECIFICATIONS AND STANDARDS REFERENCED HEREIN, PUBLISHED BY THE FOLLOWING ORGANIZATIONS, APPLY TO THE WORK ONLY TO THE EXTENT SPECIFIED BY THE REFERENCE: AMERICAN CONCRETE INSTITUTE (ACI), AMERICAN SOCIETY FOR TESTING & MATERIALS (ASTM).

1.4 SUBMITTALS:

1.4.1 PRODUCT DATA:

.1 SUBMIT CERTIFICATES OF COMPLIANCE FOR PORTLAND CEMENT AND REINFORCEMENT.

.2 SUBMIT MANUFACTURE'S TECHNICAL LITERATURE FOR ADMIXTURES AND CURING COMPOUNDS.

.3 SUBMIT MIX DESIGN SHOP DRAWINGS.

.4 SUBMIT STEEL REINFORCING SHOP DRAWINGS.

1.5 QUALITY ASSURANCE:

1.5.1 APPLICATOR QUALIFICATIONS: PROPOSED APPLICATOR SHALL HAVE AT LEAST 2 YEARS EXPERIENCE IN STRUCTURAL SHOTCRETE CONSTRUCTION AND SHALL HAVE CONSTRUCTED AT LEAST 10 SIGNIFICANT STRUCTURAL SHOTCRETE INSTALLATIONS WHICH ON INVESTIGATION, HAVE BEEN FOUND TO BE COMPLETED IN A SATISFACTORY MANNER. THE APPLICATOR SHALL BE APPROVED BY THE THE STRUCTURAL ENGINEER.

1.5.2 NOZZLEMAN QUALIFICATIONS: NOZZLEMEN SHALL BE CERTIFIED IN ACCORDANCE WITH ACI 506.3R-82 FOR THE TYPE OF SHOTCRETE APPLICATION REQUIRED ON THE JOB, OR AS APPROVED BY ENGINEER.

1.5.3 MIX DESIGN:

.1 ACCURATELY CONTROL PROPORTIONS OF WATER TO PORTLAND CEMENT TO PRODUCE THOROUGH AND UNIFORM HYDRATION OF THE SHOTCRETE THAT, WHEN APPLIED FORMS A HOMOGENEOUS MASS CONTAINING NEITHER SAGS NOR DRY SAND FORMATIONS.

.2 SHOTCRETE SHALL COMPLY WITH I.B.C. SECTION 1913.

.3 SHOTCRETE SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH (f'c) OF 4,000 PSI.

.4 MIX SHALL CONTAIN NOT MORE THAN 4.5 CUBIC FEET OF DRY SAND PER 94 LBS SACK OF PORTLAND CEMENT. MAXIMUM TOTAL CHLORIDE ION CONTENT IN THE SHOTCRETE MIX, INCLUDING AGGREGATE, CEMENT, WATER, AND ADMIXTURES SHALL NOT EXCEED LIMITS RECOMMENDED IN ACI 201.2R-77.

PART 2 - EXECUTION

2.2 SPLICES:

.1 CONTACT SPLICES MAY BE USED FOR #5 BARS & SMALLER. LARGER BARS REQUIRE NON-CONTACT SPACING OF 2-1/2 INCHES OR 3 BAR DIA. (WHICHEVER IS GREATER) AT SPLICES.

.2 BAR SPLICES SHALL BE PER S1.1

2.2.1 APPROVAL: OBTAIN INSPECTION AND APPROVAL AS REQUIRED BY NOTE SECTION 3.4 BEFORE SHOTCRETE IS PLACED.

2.3 REQUIRED COMPRESSIVE STRENGTHS: MINIMUM COMPRESSIVE STRENGTH OF 2,800 PSI FOR 14 DAY CYLINDER & 4,000 PSI FOR 28 DAY CYLINDER.

STRUCTURAL STEEL

1. WORK SHALL BE IN ACCORDANCE WITH THE LRFD STEEL CONSTRUCTION MANUAL, 2ND EDITION.
2. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH THE A.W.S. STANDARD AWS D1.1-98 "STRUCTURAL WELDING CODE - STEEL".
3. HIGH STRENGTH BOLTS, NUTS AND WASHERS USED FOR STRUCTURAL CONNECTIONS SHALL BE FRICTION OR BEARING TYPE CONFORMING TO "SPECIFICATIONS FOR STRUCTURAL JOINT USING ASTM A325 BOLTS" OF THE RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS OF THE ENGINEERING FOUNDATION, LATEST EDITION.
4. STRUCTURAL STEEL: ALL W SHAPES BEAMS ARE ASTM A992-GRADE 50 KSI U.N.O. ON PLAN
5. TS SHAPES (STEEL TUBES): ASTM A500, GRADE B. Fy = 46.
6. PIPE SECTIONS: ASTM A53 GRADE B. Fy = 35 KSI.
7. MACHINE BOLTS: ASTM A307.
8. HIGH STRENGTH BOLTS: ASTM A325, U.N.O.
9. STUD ANCHORS: NELSON SELF FLUXED ANCHORS OR EQUAL.
10. WELDING ELECTRODES: E-70 SERIES
11. STEEL SHALL BE IDENTIFIED BY MILL CERTIFICATES.
12. BOLT HOLES: STANDARD BOLT HOLES.

    FOR BEAM AND GIRDERS: 1/16" OVERSIZE BOLT HOLES FOR BOLTS LESS THAN OR EQUAL TO 1" DIAMETER. 1/8" OVERSIZE BOLT HOLES FOR BOLTS GREATER THAN 1" DIAMETER.

    FOR BASE PLATES: 5/16" OVERSIZE BOLT HOLES FOR BOLTS LESS THAN OR EQUAL TO 1" DIAMETER. 1/2" OVERSIZE BOLT HOLES FOR BOLTS GREATER THAN 1" DIAMETER.
13. WELDING: "SMAW" OR "FCAW" PROCESSES USING CERTIFIED WELDERS.
14. BASE PLATES: BEDDED ON 2 INCH MINIMUM DRYPACK GROUT, U.O.N.
15. FIELD WELDING AND HIGH STRENGTH BOLTING SHALL BE CONTINUOUSLY INSPECTED BY A REGISTERED DEPUTY INSPECTOR.
16. FILLER METAL USED IN ARC WELDING SHALL BE IN ACCORDANCE WITH TABLE 4.1.1 OF "STRUCTURAL WELDING CODE - STEEL" (AWS D1.1-90). THE MINIMUM TENSILE STRENGTH FOR FILLER METAL SHALL BE 70 KSI.
17. ALL FULL PENETRATION BUTT WELDS ARE REQUIRED TO HAVE ULTRASONIC TESTING (UT) PERFORMED BY A CERTIFIED TESTING INSPECTION LABORATORY.
18. WELDS IDENTIFIED AS REQUIRING CONTINUOUS OR PERIODIC SPECIAL INSPECTION NEED NOT HAVE A SPECIAL INSPECTION WHEN WELDING IS DONE IN AN APPROVED FABRICATORS SHOP. HOWEVER, THE APPROVED FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE IN ACCORDANCE WITH U.B.C. SECTION 3016(1).
19. STRUCTURAL STEEL SHOP DRAWINGS: TO BE REVIEWED BY THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE FABRICATION.
20. EXPOSED STRUCTURAL STEEL AND MISCELLANEOUS STEEL: HOT DIP GALVANIZED.

REINFORCED MASONRY (SPECIAL INSPECTION REQ'D PER U.B.C.)

1. CONCRETE BLOCK UNITS SHALL BE MEDIUM WEIGHT AND COMPLY WITH ASTM C90, GRADE 'N', AND SHALL ALSO COMPLY WITH U.B.C. STANDARD NO.21-4 FOR f'm = 1500 PSI. OPEN END UNITS SHALL BE GROUTED SOLID AND NEED NOT COMPLY WITH THE WEB THICKNESS REQUIREMENTS OF TABLE 21-A-C OF U.B.C. STANDARD NO.21-4. PROVIDE DEEP CUT BOND BEAM UNITS WHERE HORIZONTAL REINFORCEMENT OCCURS.
2. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AFTER 28 DAYS. RECOMMENDED MIXING PROPORTIONS BY VOLUME:
   PORTLAND CEMENT: 1 PART
   SAND (BASED ON DAMP LOOSE CONDITION): NOT LESS THAN 2 1/4 AND NOT MORE THAN 3 TIMES THE SUM OF THE SEPARATE VOLUMES OF CEMENTITIOUS MATERIALS.
   LIME: OVER 1/4 TO 1/2 PARTS BY VOLUME.
3. COARSE GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AFTER 28 DAYS. RECOMMENDED MIXING PROPORTIONS BY VOLUME:
   PORTLAND CEMENT: 1 PART
   SAND (BASED ON DAMP LOOSE CONDITION): 2 1/4 TO 3 TIMES THE SUM OF THE VOLUMES OF THE CEMENTITIOUS MATERIALS.
   PEA GRAVEL (BASED ON DAMP LOOSE CONDITION): 1 TO 2 TIMES THE SUM OF THE VOLUMES OF THE CEMENTITIOUS MATERIALS.
4. REINFORCING STEEL SHALL COMPLY WITH ASTM A615, GRADE 40. WELDED REINFORCEMENT SHALL COMPLY WITH ASTM A706 OR VERIFIED EQUIVALENT.
5. MATERIAL QUALITY CONTROL: PRIOR TO DELIVERY OF MASONRY MATERIALS TO THE JOB SITE, DELIVER TO THE ARCHITECT A LETTER CERTIFYING THAT ALL SUCH MATERIALS TO BE DELIVERED TO THE JOB SITE ARE IN STRICT CONFORMANCE WITH THE PROVISIONS OF THIS SECTION.
6. PROVIDE SPECIAL INSPECTION AS PER SECTION 1704.5 (1701.5(7)) OF THE U.B.C. FOR ALL MASONRY WORK.
7. UNLESS OTHERWISE DETAILED: LAP #4 AND #5 REINFORCEMENT BARS A MINIMUM OF 40 DIAMETERS, LAP #6 AND LARGER REINFORCEMENT BARS A MINIMUM OF 60 DIAMETERS. STAGGER ALL HORIZONTAL LAP SPLICES.
8. UNLESS OTHERWISE DETAILED: FOR ALL HORIZONTAL REINFORCING, PROVIDE A STANDARD 90 DEGREE HOOK AT THE END OF ALL WALLS. HOOK THE REINFORCING INTO THE INTERSECTING WALL OR HOOK INTO THE LAST VERTICAL CELL WHERE NO INTERSECTING WALL OCCURS.
9. PROVIDE 3/4" MINIMUM OR ONE BAR DIAMETER, WHICHEVER IS GREATER, GROUT BETWEEN THE FACE OF A REBAR AND ANY INTERIOR SURFACE OF THE MASONRY.
10. ALL VERTICAL REINFORCING SHALL BE IN ONE PIECE FROM FOOTING TO ROOF, OR FROM FOOTING TO FLOOR AND FROM FLOOR TO ROOF AND SHALL BE SECURED AGAINST DISPLACEMENT PRIOR TO GROUTING BY WIRE POSITIONERS OR OTHER SUITABLE DEVICES NOT EXCEEDING 200 BAR DIAMETER INTERVALS NOR 10'-0" MAXIMUM ON CENTER. SPLICING OF REINFORCEMENT WILL NOT BE ACCEPTED UNLESS APPROVED PRIOR TO THE START OF ANY MASONRY WORK.
11. ALL CELLS SHALL BE GROUTED SOLID. GROUT LIFTS SHALL NOT EXCEED 6'-0" IN HEIGHT. CLEANOUT HOLES SHALL BE PROVIDED AT THE BASE OF ALL GROUTED CELLS FOR GROUT LIFTS OVER 5'-0" HIGH. GROUT-AID SHALL BE ADDED TO THE MIX FOR GROUT LIFTS OVER 5'-0" HIGH.
12. GROUT CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE POUR ½" BELOW THE UPPERMOST UNIT.
13. GROUT SHALL BE CONSOLIDATED BEFORE LOSS OF PLASTICITY USING MECHANICAL VIBRATING EQUIPMENT TO MINIMIZE VOIDS DUE TO WATER LOSS. RODDING WILL NOT BE ACCEPTED.
14. PROVIDE RTICAL CONTROL JOINTS IN ALL MASONRY WALLS OVER 60 FT. IN LENGTH. IF THE LOCATIONS OF THE CONTROLVE JOINTS ARE NOT LOCATED ON THE CONTRACT DOCUMENTS THEN, THE CONTRACTOR SHALL SPACE THE JOINTS AT 3 X THE AVERAGE MAXIMUM HEIGHT OF THE WALL AND PROVIDE THE ARCHITECT WITH A SHOP DRAWING SUBMITTAL INDICATING THE PROPOSED LOCATIONS FOR APPROVAL. THE CONTROL JOINT SHALL BE IN ACCORDANCE WITH THE TYPICAL DETAIL ON SHEET S1.2.

REINFORCED CONCRETE (CONTINUOUS SPECIAL INSPECTION REQUIRED)

1. ALL CONCRETE WORK SHALL CONFORM TO THE "REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND THE "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301), LATEST APPROVED EDITIONS, WITH MODIFICATIONS AS NOTED IN THESE DRAWINGS AND SPECIFICATIONS.
2. ALL REINFORCING DETAILING SHALL CONFORM TO THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE" LATEST EDITION.
3. CONTINUOUS INSPECTION BY A REGISTERED DEPUTY INSPECTOR IS REQUIRED FOR ALL STRUCTURAL CONCRETE WORK.
4. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE ENGINEER PRIOR TO FABRICATION.
5. CONCRETE STRENGTHS: THE CONCRETE STRENGTHS SHOWN IN THE FOLLOWING TABLE ARE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS, THE AGGREGATES SHOWN ARE THE MAXIMUM SIZE (INCHES) AND THE SLUMP SHOWN IS THE MAXIMUM (INCHES).

ITEM OF CONSTRUCTION (145 P.C.F. AVG.)	STRENGTH	AGGREGATE	SLUMP
A. FOUNDATION SYSTEM	4,000 PSI	1 1/2	4
B. SLAB ON GRADE AND GRADE BEAMS	3,000 PSI	1	3

6. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60.
7. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, FLAT SHEETS.
8. MINIMUM PROTECTIVE CONCRETE COVER OF REINFORCING:
   A. ON EARTH SIDE WHEN PLACED AGAINST EARTH.....3 IN.
   B. ON EARTH SIDE WHEN FORMED.....2 IN.
   C. EXTERIOR WALL STEEL ABOVE GRADE.....1 IN.
   D. INTERIOR WALL STEEL AND SUPPORTED SLABS.....1 IN.
   E. TIED COLUMNS (TO TIES) ABOVE GRADE.....1 1/2 IN.
   F. BEAMS (TO STIRRUPS) ABOVE GRADE.....1 1/2 IN.
   G. WELDED WIRE FABRIC.....CENTER LINE OF SLAB
9. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE COLUMNS, WALLS OR SLABS UNLESS SPECIFICALLY DETAILED OR UNLESS SLEEVES ARE PROVIDED IN ACCORDANCE WITH THE TYPICAL DETAIL ON SHEET S-1.1
10. WALLS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE & SPACING.
11. SPLICE CONTINUOUS REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL ON SHEET S-1.1.
12. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL ON SHEET S-1.1.
13. REFER TO THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PIPES, DUCTS, VENTS AND SIMILAR OPENINGS.
14. REINFORCING, ANCHOR BOLTS AND ALL OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND SHALL BE INSPECTED PRIOR TO PLACING CONCRETE.
15. HOLDDOWN ANCHORS TO BE TIED IN PLACE PRIOR TO CALLING FOR FOUNDATION INSPECTION.
16. CHAMFER: 3/4" ON ALL EXPOSED CORNERS.

TIMBER

* 1. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR (NORTH) AND SHALL BE GRADE MARKED AS FOLLOWS UNLESS OTHERWISE NOTED:
   A. LIGHT FRAMING, BLOCKING AND NON BEARING
   STUDS LESS THAN 12'-0".....STANDARD OR BETTER
   B. BEARING WALLS:
   STUDS LESS THAN 9'-0".....NO. 1/ NO. 2 U.N.O.
   STUDS OVER 9'-0".....SELECT STRUCTURAL U.N.O.
   C. RAFTERS, CEILING AND FLOOR JOISTS.....NO. 1/ NO. 2 U.N.O.
   D. 4x HEADERS x 12'-0" OR LESS.....NO. 1/ NO. 2
   E. BEAMS AND STRINGERS.....NO. 1 U.N.O.
   F. 6X POSTS AND TIMBERS.....NO. 1 U.N.O.
   G. 4X POST.....NO. 1/NO. 2

   * THE MOISTURE CONTENT FOR ALL 2x SAWN WOOD LUMBER MUST NOT EXCEED 2ND LEVEL 15% (KILN-DRY). THE MOISTURE CONTENT FOR ALL OTHER SAWN WOOD LUMBER AND GLB MUST NOT EXCEED 19% PRIOR TO BUILDING FINISH ENCLOSURE OR BY ENGINEER SITE REVIEW.

2. PLYWOOD DOUGLAS FIR - U.S. DEPT. OF COMMERCE PS 1-83 (EXTERIOR GLUE)
   A. 3/8" PLYWOOD SHEAR PANELS.....GROUP 1, CD-EXPOSURE 1, INDEX 24/0
   B. 1/2" PLYWOOD ROOF SHEATHING.....GROUP 1, CD-EXPOSURE 1, INDEX 32/16
   C. 3/4" PLYWOOD FLOOR SHEATHING.....GROUP 1, CD-EXPOSURE 1, INDEX 32/16
3. PROVIDE MINIMUM NAILING IN ACCORDANCE WITH THE NAILING SCHEDULE, 3/S1.2
4. PROVIDE A 3x MEMBER FOR ALL PLYWOOD EDGE NAILING SPACING CLOSER THAN 3" ON CENTER.
5. SILL PLATES ON CONCRETE SHALL BE DOUGLAS FIR, PRESSURE TREATED, WITH 5/8" DIA. x 10" ANCHOR BOLTS AT 32"o/c MAXIMUM. CLOSER SPACING IS REQUIRED AT SHEAR WALLS. PROVIDE MINIMUM TWO ANCHOR BOLTS PER WALL WITH A MINIMUM OF 7 DIAMETERS AND 12" MAXIMUM END DISTANCE.
6. TOP PLATES OF ALL STUD WALLS SHALL BE DOUBLED, SAME SIZE AS STUDS. LAP PLATES 4'-0" MINIMUM WITH AT LEAST 12 - 16d NAILS @ 4" MAXIMUM ON CENTER IN THE LENGTH OF THE LAP. REFER TO SPLICE DETAIL.
7. SOLID BLOCK STUDS AT MID HEIGHT OF WALL AT CORNER & WINDOWS. WHERE STUDS ARE NOT SHEATHED ON ONE OF BOTH SIDES PROVIDE HORIZONTAL ROWS OF SOLID BLOCKING BETWEEN STUDS AT 48" O.C. MAX.
8. PLACE 2X SOLID BLOCKING BETWEEN ALL JOISTS AND RAFTERS AT SUPPORTS AND UNDER ALL PARTITIONS, TYPICAL U.N.O.
9. BOLTS SHALL HAVE 7 DIAMETER MINIMUM END DISTANCE AND 4 DIAMETER MINIMUM EDGE DISTANCE, UNLESS OTHERWISE NOTED.
10. ALL BOLTS SHALL BE FITTED WITH WASHERS. HOLES IN WOOD SHALL BE BORED WITH A BIT 1/32" TO 1/16" LARGER THAN THE BOLT.
11. STEEL FRAMING CONNECTORS SHALL BE MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY UNLESS ALTERNATE CONNECTORS HAVE BEEN APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION. CONNECTORS SUBJECTED TO CONDITIONS OF EXCESSIVE MOISTURE SHALL BE HOT-DIPPED GALVANIZED.
12. GLU-LAM BEAMS SHALL BE COMBINATION APA-EWS 28F-E1 SP/SP FOR DRY USE UNLESS OTHERWISE NOTED. THE MIN. DESIGN PROPERTIES SHALL BE Fb=2400 PSI, Fv=165 PSI, & Fc=1650 PSI, E=1.6x10⁶ PSI. MANUFACTURED IN ACCORDANCE WITH A.I.T.C. SPECIFICATION 117-84 OR APA-EWS 117/ANSI A190.1 STANDARDS.
13. AN A.I.T.C. CERTIFICATE OF COMPLIANCE SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT PRIOR TO THE ERECTION OF GLU-LAM BEAMS. SUBMIT COPIES OF THE CERTIFICATE WITH ATTACHMENTS TO THE ARCHITECT.

GENERAL NOTES

THE FOLLOWING GENERAL NOTES ARE A SUMMARY OF THE SPECIFICATIONS FOR THE CONVENIENCE OF THE CONTRACTOR. REFER TO THE SPECIFICATIONS.

GENERAL

1. CONTRACTOR SHALL VERIFY ALL DIMENSIONS BEFORE STARTING WORK AND NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES FOUND.
2. SPECIFIC CODES AND DETAILS SHALL TAKE PRECEDENCE OVER THESE GENERAL NOTES AND THE TYPICAL DETAILS ON SHEETS S1.0 TO S1.6 IN CASE OF CONFLICT.
3. WHERE NO CONSTRUCTION DETAILS ARE SHOWN OR NOTED FOR ANY PART OF THE WORK, THE DETAILS USED SHALL BE THE SAME AS FOR OTHER SIMILAR WORK, PROVIDED THAT PRIOR APPROVAL IS OBTAINED FROM THE ARCHITECT OR ENGINEER.
4. THE CONTRACTOR SHALL COORDINATE THE WORK OF ALL TRADES WITH THE STRUCTURAL REQUIREMENTS, REFER TO CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
5. THE DESIGN IS BASED ON THE 2007 CALIFORNIA BLDG. CODE (CBC). THE 2007 CBC IS BASED ON THE 2006 IBC, & REFERENCES DOCUMENTS SUCH AS ASCE 7-05, ACI 318-05, AISC 13TH. EDITION, NDS-2005. SEE ALSO CALCS. PAGE D1.
6. NEITHER THE OWNER NOR THE ARCHITECT WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE SUBCONTRACTOR SHALL DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING, AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.

DESIGN CRITERIA

1. "LATERAL FORCE REQUIREMENTS AND COMMENTARY" BY THE STRUCTURAL ENGINEERS ASSOCIATION OF CALIFORNIA, 1999 EDITION.
2. "INTERNATIONAL BUILDING CODE" BY THE INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS, 2006 EDITION.
3. "CALIFORNIA BUILDING CODE", 2007 EDITION.
4. "CALIFORNIA EXISTING BUILDING CODE", 2007 EDITION.
5. DESIGN LOADS:
   A. LIVE LOADS
   ROOF.....20 PSF
   FLOOR.....50 PSF
   CORRIDOR & STAIR.....100 PSF
   B. WIND (PER 2007 CBC).....SEISMIC CONTROLS
   C. SEISMIC

SEISMIC IMPORTANT FACTOR	I = 1.0
OCCUPANCY CATEGORY	II
MAPPED SPECTRAL RESPONSE ACCELERATION	Ss = 1.571, S1 = 0.614
SITE CLASS	D
SPECTRAL RESPONSE COEFFICIENT	SDS = 1.047 SD1 = 0.614
BASIC SEISMIC-FORCE RESISTING SYSTEM:	ORDINARY STEEL CONCENTRICALLY BRACED FRAMES
DESIGN BASE SHEAR	V = CsW = 0.322W
SEISMIC RESPONSE COEFFICIENT	Cs = (SDs/(R/I))M
RESPONSE MODIFICATION FACTOR	R = 3.25
ANALYSIS PROCEDURE USED	EQUIVALENT LATERAL FORCE ANALYSIS SECTION 12.8, ASCE 7-05

FOUNDATION

1. MAXIMUM SOIL PRESSURE: 3,000 PSF
2. FOOTINGS SHALL EXTEND A MINIMUM DEPTH OF 18" BELOW LOWEST ADJACENT FINISH GRADE AND BE FOUNDED A MINIMUM OF 12 IN. INTO COMPETENT MATERIALS.

REINFORCING STEEL

1. DETAILS OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH CHAPTER 7 OF "AMERICAN CONCRETE INSTITUTE 318" UNLESS OTHERWISE NOTED. REINFORCING STEEL DETAILING, BENDING AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
2. WELDING OF REINFORCING STEEL, IF PERMITTED BY THE ARCHITECT, SHALL BE IN ACCORDANCE WITH THE "STRUCTURAL WELDING CODE - REINFORCING STEEL" OF THE AMERICAN WELDING SOCIETY, AWS D1-4, AND SHALL BE PERFORMED BY WELDERS QUALIFIED UNDER THE PROCEDURES CONTAINED THEREIN.
3. ALL REINFORCING STEEL SHOP DRAWINGS SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER AND THE ARCHITECT PRIOR TO FABRICATION.
4. ALL REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60 FOR NO.5 AND LARGER, OTHERWISE GRADE 40. WELDED REBAR SHALL CONFORM TO ASTM 706.
5. WALLS SHALL BE DOWELED FROM SUPPORTS WITH BARS OF THE SAME SIZE AND SPACING.
6. SPLICE MINIMUM REINFORCING IN ACCORDANCE WITH THE TYPICAL DETAIL ON SHEET S-1.1
7. PROVIDE MINIMUM EMBEDMENT OF REINFORCING IN CONFORMANCE WITH THE TYPICAL DETAIL ON SHEET S-1.1
8. BAR SUPPORTS SHALL BE PROVIDED IN ACCORDANCE WITH THE PROVISIONS OF "BAR SUPPORT SPECIFICATION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
9. ALL REINFORCING STEEL, ANCHOR BOLTS AND OTHER EMBEDDED ITEMS SHALL BE SECURELY HELD IN POSITION AND INSPECTED PRIOR TO PLACING CONCRETE.

ADD TO: PTS# 182896

BUILDING DEVELOPMENT REVIEW DIVISION-STRUCTURAL
BONIFACE SIMBWA Date 07/27/09

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REGISTERED PROFESSIONAL ENGINEER RAYMOND WILLIAMS SMITH No. S-5161 STRUCTURAL STATE OF CALIFORNIA

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Gary and Mary West Senior Wellness Center

1525 Fourth Avenue
Senior Community Center

Senior Community Centers

Revision Dates
PERMIT SET 7.22.09

Issue Dates
DPR
Design Development
Plan Check 6.15.09
Bid Set
Permit Set
Construction Set

Drawing Date 6-8-09
Check By RHF
Drawn By KBM
Scale AS SHOWN
Job Number FLC09023
Sheet Number

S1.0

STEEL CONSTRUCTION - VERIFICATION AND INSPECTION REQUIREMENTS (CBC T-1704.3)

ITEM OF VERIFICATION AND INSPECTION	INPECTION FREQUENCY		REFERENCE FOR TESTING & INSPECITON CRITERIA		
	CONTINUOUS	PERIODIC	REFERENCED STANDARD	CBC REFERENCE	PROJECT SPECIFIC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:					
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.		X	APPLICABLE ASTM MATERIAL SPECIFICATIONS: AISC 360, SEC. A3.3	-	51.5, 54.2, 55.0, 56.0
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.		X			
2. INSPECTION OF HIGH-STRENGTH BOLTING:					
A. BEARING-TYPE CONNECTIONS.		X	AISC 360, SECTION M2.5	1704.33	51.5, 54.2, 55.0, 56.0
B. SLIP-CRITICAL CONNECTIONS.	X				
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:					
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.			ASTM A6 OR ASTM A568	1708.4	51.5, 54.2, 55.0, 56.0
B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS.			ASTM A6 OR ASTM A568		
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:					
A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.			AISC 360, SECTION A3.5	-	51.5, 54.2, 55.0, 56.0
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.					
5. INSPECTION OF WELDING:					
A. STRUCTURAL STEEL:					51.5, 54.2, 55.0, 56.0
1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS.	X		AWS D1.1	1704.3.1	
2) MULTIPASS FILLET WELDS.	X				
3) SINGLE-PASS FILLET WELDS ≥ 5/16".	X				
4) SINGLE-PASS FILLET WELDS ≤ 5/16".		X			
5) FLOOR AND ROOF DECK WELDS.		X			
B. REINFORCING STEEL:			AWS D1.3	-	54.2
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.		X	AWS D1.4 ACI 318: 3.5.2.	-	
2) REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT.	X				
3) SHEAR REINFORCEMENT.	X				
4) OTHER REINFORCING STEEL.		X			
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS.		X	-	1704.3.2.	54.2
A. DETAILS SUCH AS BRACING AND STIFFENING.		X			
B. MEMBER LOCATIONS.		X			
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.		X			

CONCRETE CONSTRUCTION - VERIFICATION AND INSPECTION REQUIREMENTS (CBC T-1704.4)

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	CBC REFERENCE	PROJECT SPECIFIC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.		X	ACI 318: 3.5, 7.1-7.7	1913.4	52.0, 52.1, 53.0
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5B.		X	AWS D1.4 ACI 318: 3.5.2	-	51.1
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.	X		-	1911.5	N/A
4. VERIFYING USE OF REQUIRED DESIGN MIX.		X	ACI 318: CH. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3	51.0
5. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X		ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913.10	51.0
6. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X		ACI 318: 5.9, 5.10	1913.6, 1913.7, 1913.8	51.0
7. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		X	ACI 318: 5.11, 5.13	1913.9	51.0
8. INSPECTION OF PRESTRESSED CONCRETE:					
A. APPLICATION OF PRESTRESSING FORCES.	X		ACI 318: 18.20 ACI 318: 18.18.4	-	N/A
B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM.	X				
9. ERECTION OF PRECAST CONCRETE MEMBERS.		X	ACI 318: CH.16	-	N/A
10. VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.		X	ACI 318: 6.2	-	N/A
11. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		X	ACI 318: 6.1.1	-	52.0, 52.1, 53.0

MASONRY CONSTRUCTION - VERIFICATION AND INSPECTION REQUIREMENTS (CBC T-1704.5.1)

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	CBC REFERENCE	ACI 530/ASCE 5/TMS 402	ACI 530.1/ASCE 6/TMS 602	PROJECT SPECIFIC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.						51.0, 2.1, 53.0
A. PROPORTIONS OF SITE-PREPARED MORTAR.		X	-	-	ART. 2.6A	
B. CONSTRUCTION OF MORTAR JOINTS.		X	-	-	ART. 3.3B	
C. LOCATION OF REINFORCEMENT, CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		X	-	-	ART. 3.4, 3.6A	
D. PRESTRESSING TECHNIQUE.		X	-	-	ART. 3.6B	
E. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		X	-	-	ART. 2.4B, 2.4H	
2. THE INSPECTION PROGRAM SHALL VERIFY:						
A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.		X	-	-	ART. 3.3G	
B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.		X	-	SEC. 1.2.2(E), 2.1.4, 3.1.6	-	
C. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.		X	-	SEC. 1.13	ART. 2.4, 3.4	
D. WELDING OF REINFORCING BARS.	X		-	SEC. 2.1.10.7.2, 3.3.3.4(B)	-	
E. PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40° F) OR HOT WEATHER (TEMPERATURE ABOVE 40° F).		X	SEC. 2104.3, 2104.4	-	ART. 1.8C, 1.8D	
F. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	X		-	-	ART. 3.6B	
3. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:						
A. GROUT SPACE IS CLEAN.		X	-	-	ART. 3.2D	
B. PLACEMENT OF REINFORCEMENT AND CONNECTORS AND PRESTRESSING TENDONS AND ANCHORAGES.		X	-	SEC. 1.13	ART. 3.4	
C. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS.		X	-	-	ART. 2.6B	
D. CONSTRUCTION OF MORTAR JOINTS.		X	-	-	ART. 3.3B	
4. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.	X		-	-	ART. 3.5	
A. GROUTING OF PRESTRESSING BONDED TENDONS.	X		-	-	ART. 3.6C	
5. PREPARATION OF ANY REQUIREDE GROUT SPECIMENS, MORTAR AND SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	X		SEC. 2105.2.2, 2105.3	-	ART. 1.4	
6. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.		X	-	-	ART. 1.5	

STATEMENT OF SPECIAL INSPECTIONS:

SOILS - VERIFICATION AND INSPECTION REQUIREMENTS (CBC T-1704.7)

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	PROJECT SPECIFIC REFERENCE
1. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		X	REFER TO GEOTECHNICAL REPORT PROPOSED WELLNESS CENTER No. GNE 10200071.02 BY CHRISTIAN WHEELER ENGINEERING DATED JUNE 3, 2009 MAX SOIL PRESSURE 5000 PSF
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		X	
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.		X	
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	X		
5. PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		X	

STRUCTURAL OBSERVATIONS AND SPECIAL INSPECTION

1. THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE STRUCTURAL DESIGN SHALL SUBMIT A STATEMENT IN WRITING TO THE BUILDING OFFICIAL, STATING THAT DURING THE CONSTRUCTION OF THIS STRUCTURE, SITE VISITS HAVE BEEN PERFORMED TO OBSERVE GENERAL COMPLIANCE WITH THE APPROVED STRUCTURAL PLANS, SPECIFICATIONS AND CHANGE ORDERS. FURTHER, THE STATEMENT SHALL, IN DETAIL, NOTE HOW ANY DEFICIENCIES HAVE BEEN CORRECTED.
2. THE GENERAL CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY FOR CONSTRUCTION OF ITEMS LISTED FOR SPECIAL INSPECTION PRIOR TO COMMENCEMENT OF WORK.
3. STRUCTURAL VISUAL OBSERVATION MUST BE PROVIDED AT THE SIGNIFICANT ITEMS OF CONSTRUCTION. STRUCTURAL OBSERVATION DOES NOT INCLUDE OR WAIVE THE INSPECTIONS REQUIRED BY SECTION 1701 OR OTHER SECTIONS OF THE CODE. AS A MINIMUM, THIS STRUCTURE REQUIRES SITE OBSERVATIONS BEFORE THE FOLLOWING HAS BEEN STARTED:
   A. POURING THE INITIAL FOOTINGS / PILE CAPS.
   B. BEFORE POURING THE 1ST FLOOR CONCRETE DECK.
   C. DURING OR IMMEDIATELY AFTER COMPLETION OF THE FOLLOWING CRITICAL DETAILS:
      1. PRIOR TO PLACING CONCRETE FOR THE 1ST TO 2ND FLOOR SPECIAL MOMENT RESISTING FRAME COLUMNS.
      2. PRIOR TO PLACING CONCRETE FOR THE ENTIRE 2ND LEVEL.
4. THE CONSTRUCTION MATERIALS TESTING LABORATORY MUST BE APPROVED BY THE CITY OF SAN DIEGO DEVELOPMENT SERVICES, FOR TESTING OF MATERIALS, SYSTEMS, COMPONENTS AND EQUIPMENTS.
5. FABRICATOR SHALL BE REGISTERED AND APPROVED BY THE CITY OF SAN DIEGO DEVELOPMENT SERVICES FOR FABRICATION OF MEMBERS AND ASSEMBLIES ON THE PREMISES OF THE FABRICATOR'S SHOP.
6. FABRICATOR SHALL SUBMIT AN "APPLICATION TO PERFORM OFF-SITE FABRICATION" TO THE INSPECTION SERVICES DIVISION FOR APPROVAL PRIOR TO COMMENCEMENT OF FABRICATION.
7. FABRICATOR SHALL SUBMIT A "CERTIFICATE OF COMPLIANCE FOR OFF-SITE FABRICATION" TO THE INSPECTION SERVICES DIVISION PRIOR TO ERECTION OF FABRICATED ITEMS AND ASSEMBLIES.
8. A "PROPERTY OWNER'S FINAL REPORT FORM" FOR WORK REQUIRED TO HAVE SPECIAL INSPECTIONS, TESTING AND STRUCTURAL OBSERVATIONS MUST BE COMPLETED BY THE PROPERTY OWNER, PROPERTY OWNER'S AGENT OF RECORD, ARCHITECT OF RECORD OR ENGINEER OF RECORD AND SUBMITTED TO THE INSPECTION SERVICES DIVISION.
9. FOR SHOTCRETE CONSTRUCTION, A TEST PANEL SHALL BE SHOT, CURED, CORED OR SAWN, EXAMINED AND TESTED PRIOR TO COMMENCEMENT OF THE PROJECT.
10. SPECIAL INSPECTION IS REQUIRED DURING TAKING TEST SPECIMENS AND PLACING CONCRETE.
11. SPECIAL INSPECTION IS REQUIRED PRIOR TO AND DURING PLACEMENT OF CONCRETE AROUND THE BOLTS USING THE ALLOWABLE VALUES FOR BOLTS INSTALLED WITH SPECIAL INSPECTION.
12. SPECIAL INSPECTION IS REQUIRED DURING STRESSING AND GROUTING OF PRESTRESSED CONCRETE.
13. SPECIAL INSPECTION IS REQUIRED VERIFYING THE REINFORCING PRIOR TO CLOSING THE FORMS OR DELIVERY OF CONCRETE TO THE JOB SITE.

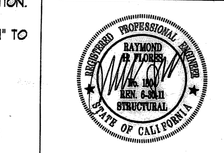
STRUCTURAL TESTING FOR SEISMIC RESISTANCE

1. FOR SEISMIC DESIGN CATEGORY AND OCCUPANCY CATEGORY AS LISTED ON GENERAL NOTES SHEET S1.0 THE FOLLOWING TESTING, CERTIFICATIONS AND INSPECTIONS SHALL BE PROVIDED FOR THIS PROJECT:
2. MASONRY -
   A. CERTIFICATES OF COMPLIANCE USED IN MASONRY CONSTRUCTION SHALL BE VERIFIED PRIOR TO CONSTRUCTION.
   B. VERIFICATION OF f'm AND f'acc SHALL BE PROVIDED PRIOR TO CONSTRUCTION.
3. REINFORCING AND PRESTRESSING STEEL -
   A. CERTIFIED MILL TEST REPORTS SHALL BE PROVIDED FOR ALL ASTM A615 REINFORCING PROVIDED FOR SPECIAL MOMENT RESISTING FRAMES AND CONTINOUS BOUNDRY/DRAG BAR REINFORCING IN THE SLAB ALONG MOMENT FRAME DRAG LINES.
   B. FOR ANY ASTM A615 REINFORCING STEEL TO BE WELDED, CHEMICAL TESTS SHALL BE PREFORMED TO DETERMINE WELDABILITY IN ACCORDANCE WITH ACI 318-3.5.2.
4. STRUCTURAL STEEL - THE TESTING CONTAINED IN THE QUALITY ASSURANCE PLAN SHALL BE AS REQUIRED BY AISC 341 AND THE ADDITIONAL REQUIREMENTS PROVIDED IN THESE PLANS AND PROJECT SPECIFICATIONS. BASE METAL THICKER THAN 1.5 INCHES SHALL BE ULTRASONICALLY TESTED FOR DISCONTINUITIES BEHIND AND ADJACENT TO WELDS AFTER JOINT COMPLETION. ACCEPTANCE CRITERIA ESTABLISHED BY ASTM A435 OR A898 AND THE PROJECT SPECIFICATIONS.
5. MECHANICAL & ELECTRICAL EQUIPMENT - THE APPLICABLE SEISMIC DESIGN CRITERIA ARE PROVIDED ON SHEET S1.0 OF THESE CONSTRUCTION DOCUMENTS. THE MANUFACTURER AND REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE OF THE DESIGNATED SEISMIC SYSTEM COMPONENTS AND MOUNTING SYSTEMS OR ANCHORAGE SHALL TEST & ANALYZE THESE COMPONENTS AND SUBMIT CERTIFICATE OF COMPLIANCE FOR REVIEW AND ACCEPTANCE TO THE PROJECT ENGINEER OF RECORD AND THE BUILDING OFFICIAL. QUALIFICATION SHALL BE BY AN ACTUAL TEST ON A SHAKE TABLE, BY 3-D SHOCK TESTS, BY AN ANALYTICAL METHOD USING DYNAMIC CHARACTERISTICS AND FORCES, BY THE USE OF EXPERIENCE DATE OR BY A MORE RIGOROUS ANALYSIS PROVIDING FOR EQUIVALENT SAFETY.

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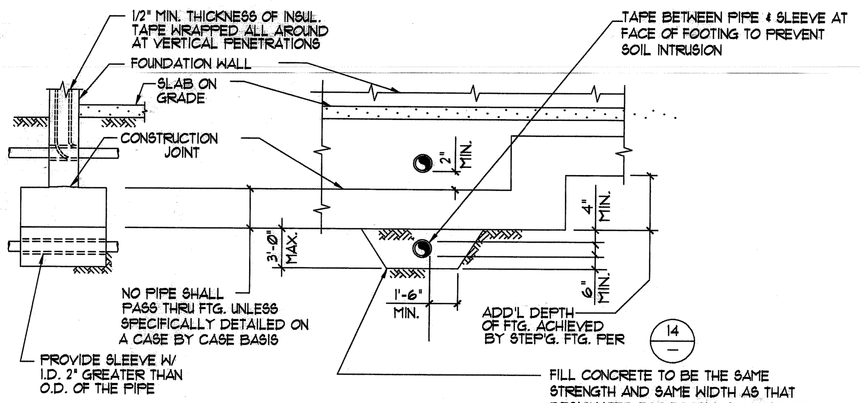
Senior Community Centers

Revision Dates

1	PERMIT SET 7.22.09
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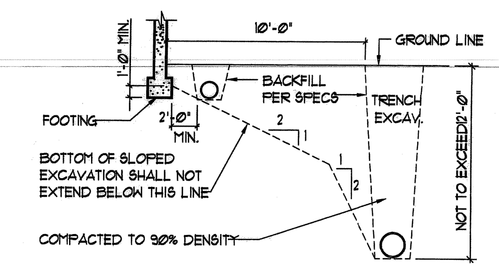
Issue Dates

DPR	.
Design Development	.
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Bid Set	.
Permit Set	.
Construction Set	.
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	**S1.0A**



DETAIL OF PIPE SLEEVE AT CONTINUOUS FOOTING.

13



NOTE :
MAINTAINING STABILITY OF SIDES OF EXCAVATION BY TEMPORARY SHORING SHEATHING OR BY PROVIDING PROTECTIVE COVERING IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

EXCAVATIONS PARALLEL TO WALL FOOTING

9

TENSION LAP SPLICES
FOR GRADE 60 BARS f'c = 3000

BAR SIZE	BOTTOM BARS		TOP BARS	
	A	B	A	B
#3	17	22	22	28
#4	22	29	29	37
#5	28	36	36	47
#6	33	43	43	56
#7	48	63	63	81
#8	55	72	72	93
#9	62	81	81	105
#10	70	91	91	118
#11	78	101	101	131

(1) LAP SPLICES SHALL BE CLASS B SPLICES.
(2) TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THE REINFORCEMENT. MULTIPLE HORIZONTAL BARS IN A SINGLE VERTICAL PLANE SUCH AS HORIZONTAL BARS IN WALLS TIES ARE NOT TOP BARS.
(3) BARS SHALL BE SPACED A MINIMUM OF 3 BAR DIAMETERS ON CENTER IN ORDER TO USE THE TABULATED SPLICE LENGTHS.

REINFORCING BAR SPLICES

5

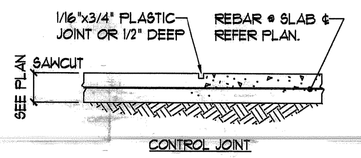
MINIMUM STRAIGHT TENSION DEVELOPMENT
FOR GRADE 60 BARS f'c = 3000

BAR SIZE	BOTTOM BARS	TOP BARS
#3	15	19
#4	17	22
#5	21	27
#6	25	33
#7	29	38
#8	33	43
#9	38	50
#10	48	63
#11	60	77
#14	81	105
#18	119	154

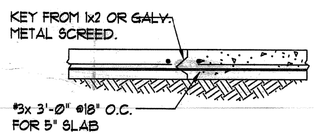
(1) BARS SHALL BE SPACED A MINIMUM OF 4 BAR DIAMETERS ON CENTER TO USE THE TABULATED DEVELOPMENT LENGTHS.
(2) FOR DEFINITION OF LAP BARS SEE 5/-

REINFORCING BAR EMBEDMENT

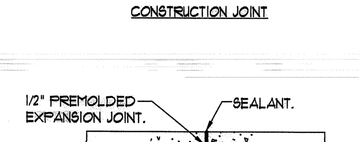
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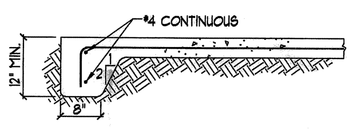
CONTROL JOINT



CONSTRUCTION JOINT



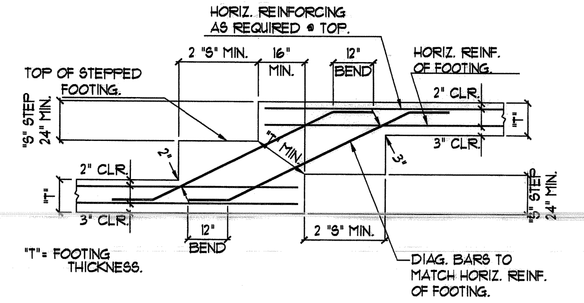
EXPANSION JOINT



EDGE TURN DOWN

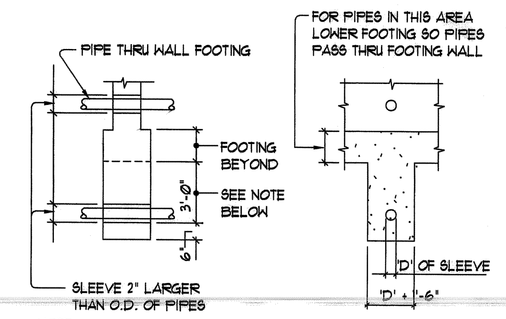
SLAB ON GRADE JOINTS

19



STEPPED FOOTING

14



NOTE :
FOR PIPES 3'-0" OR LESS BELOW FOOTING, PROVIDE SLEEVE AND CONCRETE AS SHOWN: MORE THAN 3'-0" COMPACT BACKFILL OVER PIPE TO 92% PROCTOR DENSITY U.N.O.

EXCAVATION PERPENDICULAR TO WALL FOOTING / GRADE BEAM

10

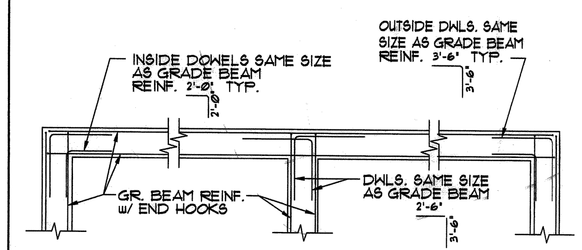
'ld' TENSION DEVELOPMENT (EMBEDMENT) LENGTH FOR STANDARD END HOOKS
(GRADE 60 BARS–NORMAL WEIGHT CONCRETE–GENERAL USE)



BAR SIZE	f'c=3000 psi	f'c=4000 psi	f'c=5000 psi
	ldh	ldh	ldh
# 3	6"	6"	6"
# 4	8"	7"	6"
# 5	10"	9"	8"
# 6	1'-0"	10"	9"
# 7	1'-2"	1'-0"	11"
# 8	1'-4"	1'-2"	1'-0"
# 9	1'-6"	1'-3"	1'-2"
# 10	1'-8"	1'-5"	1'-4"
# 11	1'-10"	1'-7"	1'-5"

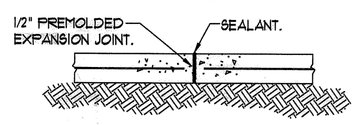
NOTE: (*) WHEN EITHER SIDE OR END COVER IS SMALLER THAN THESE NUMBERS MULTIPLY 'ldh' BY 1.4 .

6



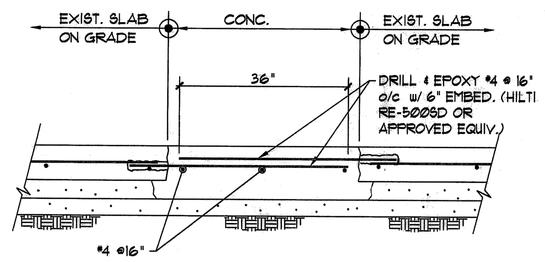
REINF. @ INTERSECTIONS OF WALLS & FOOTINGS

2



TYP. SHEAR WALL TO (E) FOOTING ANCHORAGE

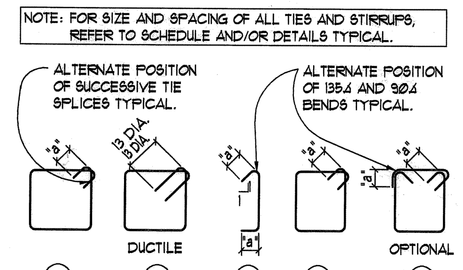
15



TYP. SLAB-ON-GRADE INFILL

11

NOTE: FOR SIZE AND SPACING OF ALL TIES AND STIRRUPS, REFER TO SCHEDULE AND/OR DETAILS TYPICAL.

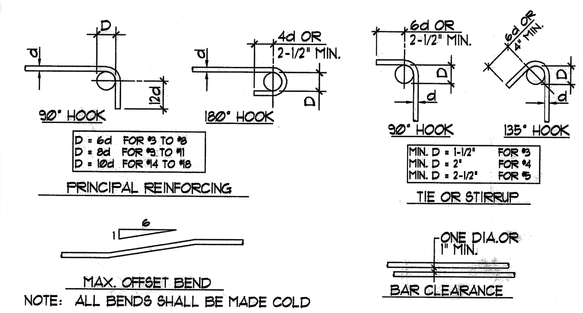


TYPICAL COLUMN/PIER TIES
BEAM STIRRUPS

DIMENSION 'a' FOR #3 TIES = 5"
FOR #4 TIES = 5½"
FOR #5 TIES = 6"

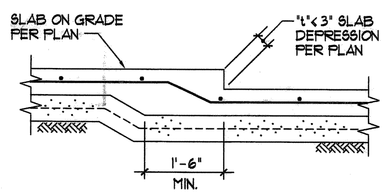
TYPICAL COLUMN TIES AND BEAM STIRRUPS

7



BAR BENDS

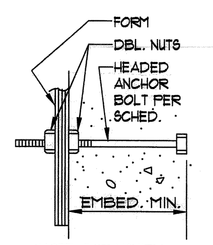
3



TYPICAL DEPRESSED SLAB DETAILS

20

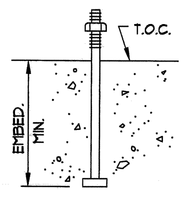
SCHEDULE		
BOLT SIZE	EMBED.	REMARKS
1/2"	4"	
5/8"	4"	
3/4"	5"	
7/8"	6"	
1"	7"	
1-1/8"	8"	
1-1/4"	9"	



*NOTE: FOR MASONRY EMBED BOLTS, SEE 2@ S1.2

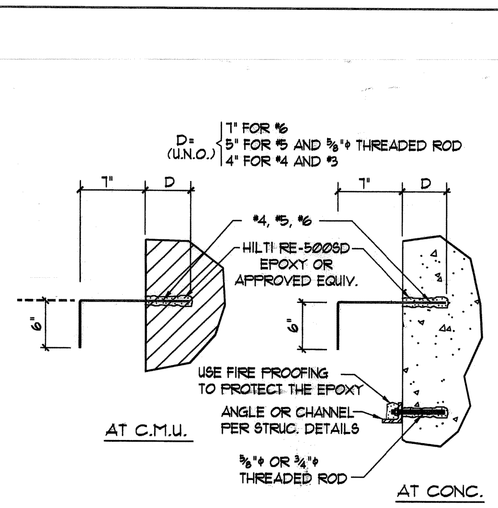
BOLT EMBEDMENT AT CONCRETE WALL

SCHEDULE		
SIZE	EMBED.	REMARKS
3/4" DIA.	9"	
1" DIA.	12"	
1-1/2" DIA.	15"	



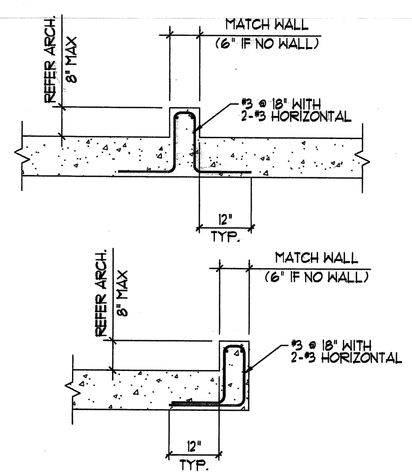
FOUNDATION HEADED ANCHOR BOLTS

16



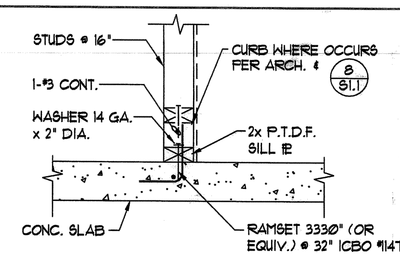
TYP. DRILL & EPOXY DETAIL

12

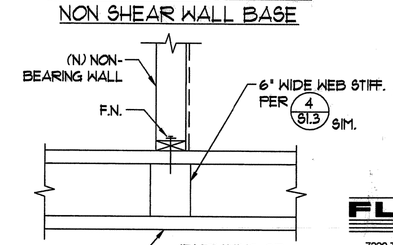


TYPICAL CURB SECTION

8



TYP. DET. @ NON BEARING NON SHEAR WALL BASE

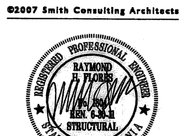


4

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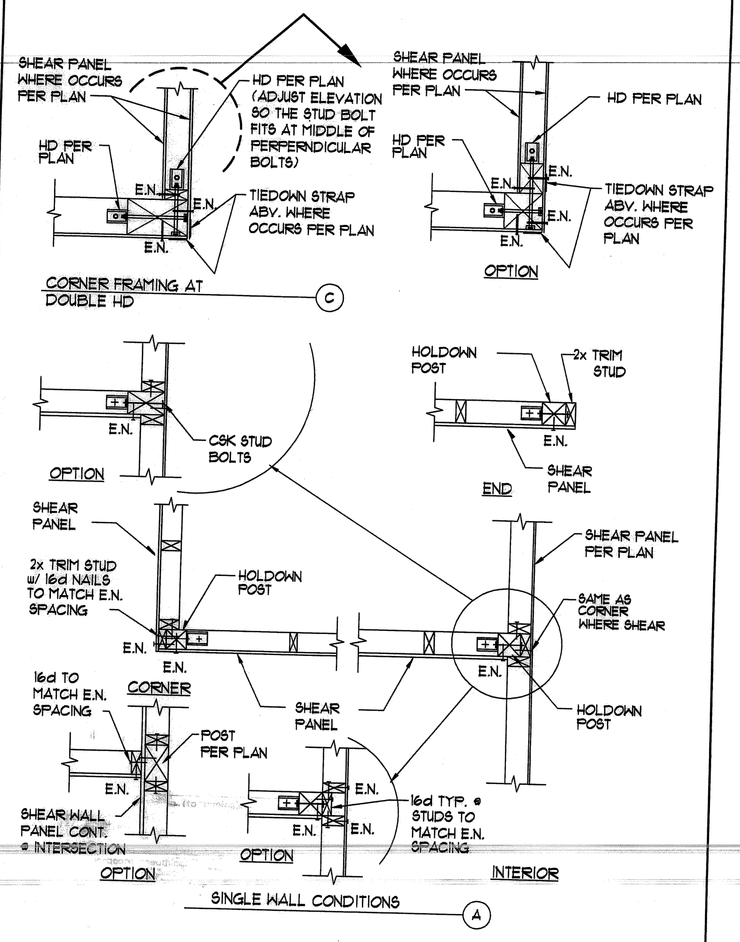
Gary and Mary West Senior Wellness Center

1525 Fourth Avenue
Senior Community Center

Senior Community Centers

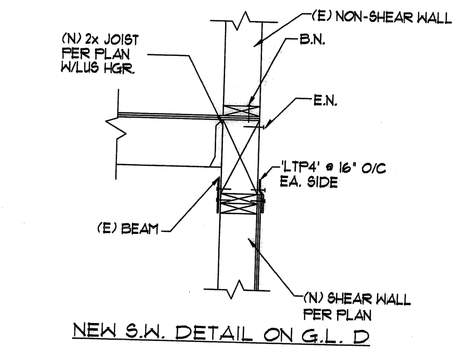
Revision Dates	
1	PERMIT SET 7.22.09

Issue Dates	
DPR	
Design Development	
Plan Check	6.15.09
Bid Set	
Permit Set	
Construction Set	
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	S1.1

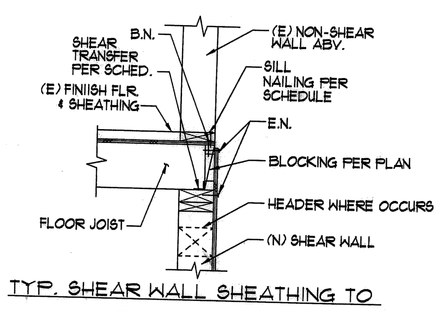


TYP. STUD INTERSECTIONS AT SHEAR WALLS

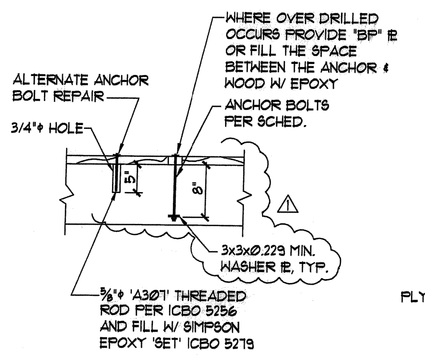
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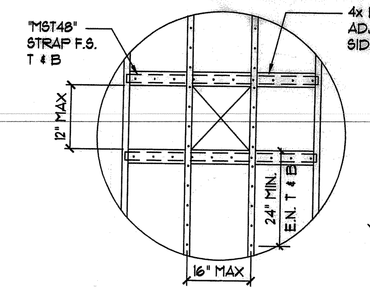


NEW S.W. DETAIL ON G.L. D

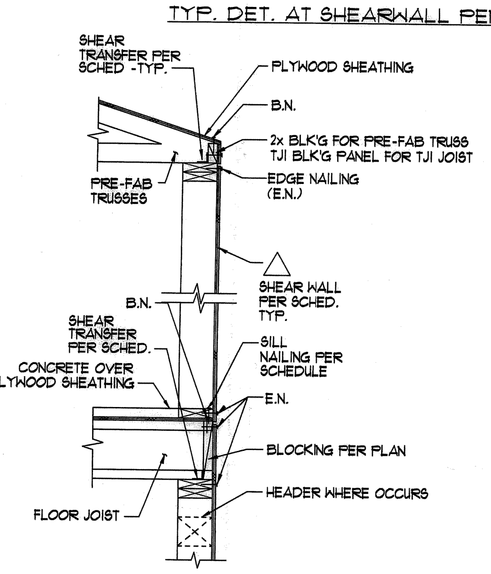


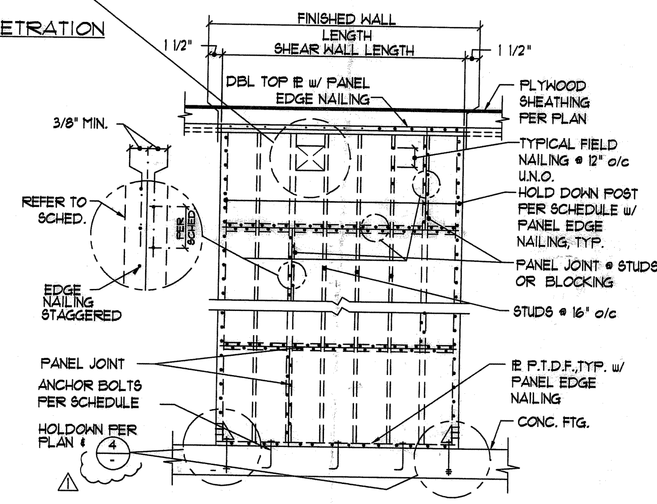
TYP. SHEAR WALL SHEATHING TO TOP PLATE





TYP. DET. AT SHEARWALL PENETRATION



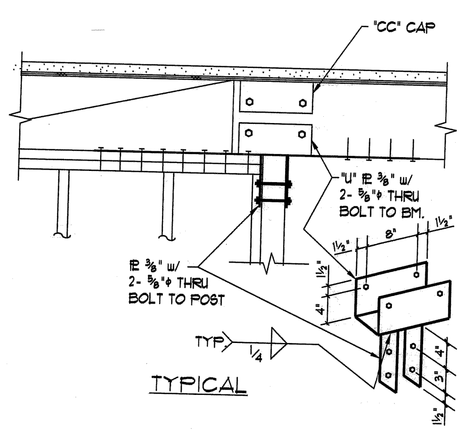


NOTES:

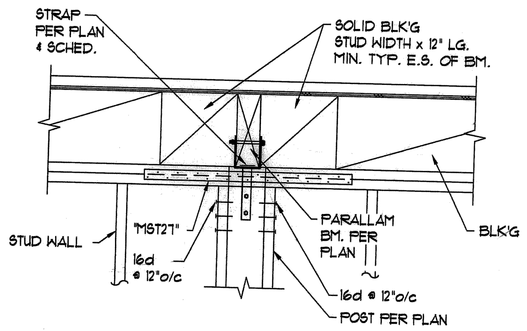
1. FIELD NAILS @ 12" o.c. ALL PANEL EDGE'S BLK'ED
2. ALLOWABLE LOADS ARE BASED ON COMMON NAILS.
3. 3 ANCHOR BOLTS MIN. PER SHEAR WALL SILL PLATES.
4. 20d NAILS SHALL BE INSTALLED IN PRE-DRILLED HOLES.
5. PROVIDE 2-11/16" LSL RIM JOIST FOR SILL NAILING LESS THAN OR EQUAL TO 3"
6. PLACE A35# OR LTP4 OVER PLYWOOD. PROVIDE 8dx2½" COMMON NAILS IN LIEU OF SIMPSON NAILS.
7. PROVIDE A.B AT CONCRETE FTG. PER SCHED.
8. SEE DETAIL 18/S1.2 FOR WINDOW OPENING STRAP
9. FOR ALT. SILL LAG'S, USE 3" MIN. EMBEDMENT INTO LSL, PSL OR SAWN SOLID BLOCKING OR RIM (NOT NCLUDING PLYWOOD)
10. ALL NAILS, LAGS, AND HARDWARE IN CONTACT WITH FIRE TREATED WOOD NEEDS TO BE HOT DIPPED GALVANIZED.
11. ALL RIM JOIST OR BLOCKING CAN BE SUBSTITUTED BY TIMBER STRAND.

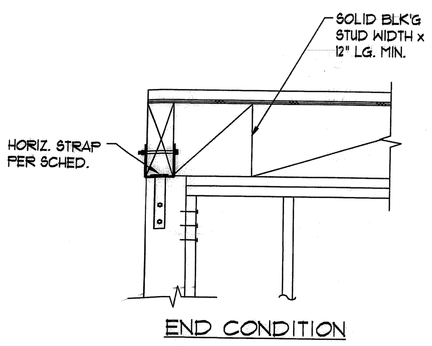
SHEAR WALL SCHEDULE (8) (12)

MARKS	SHEATHING MATERIAL (INDEX)	ALLOW. LOAD (2)	MINIMUM FRAMING UNLESS OTHERWISE NOTED							EDGE NAIL (E.N.)	FIELD NAIL (F.N.) (1)	ANCHOR BOLT (3)(7)	SILL (4)(5) NAILING	ALTERNATE SILL LAG'S BOLTS (9)	SHEAR TRANSFER NAILING (6)	REMARKS
			TOP @	BOTT. @	SILL @ TO CONC.	SILL @ TO WOOD	FIELD STUD	PANEL JOINT STUD	PANEL JOINT BLK'G.							
1	3/8 STRUCT I (24/0)	280	2x	2x	2x	2x	2x	2x	2x	8d @ 6"	8d @ 12"	5/8"ø A.B. x 12" @ 48" O.C.	16d @ 6"	3/8"ø @ 16"	A35 @ 16" O.C.	
2	3/8 STRUCT I (24/0)	430	2x	2x	3x	2x	2x	3x	3x	8d @ 4"	8d @ 12"	5/8"ø A.B. x 12" @ 32" O.C.	16d @ 4"	3/8"ø @ 12"	A35 @ 12" O.C.	
3	3/8 STRUCT I (24/0)	550	2x	2x	3x	2x	2x	3x	3x	8d @ 3"	8d @ 12"	5/8"ø A.B. x 12" @ 24" O.C.	16d @ 3"	3/8"ø @ 10"	A35 @ 8" O.C.	
4	3/8 STRUCT I (24/0)	730	3x	2x	3x	3x	2x	3x	3x	8d @ 2"	8d @ 12"	5/8"ø A.B. x 12" @ 16" O.C.	20d @ 2 1/4"	3/8"ø @ 8"	A35 @ 6" O.C.	



TYPICAL

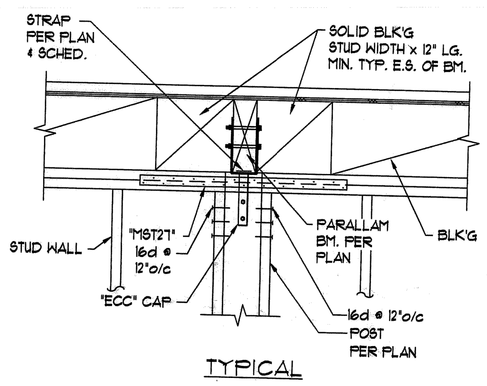




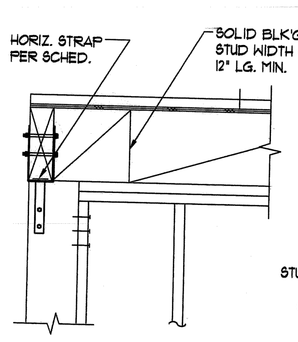
END CONDITION

HORIZONTAL STRAP SCHEDULE & DETAILS*

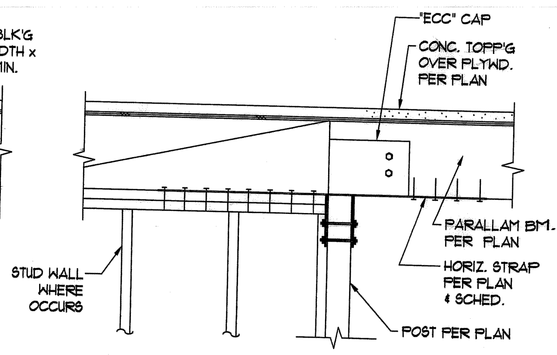
MARK	STRAP	NAILING TO EA.STRAP	BOLTS TO EA. BEAM	CAPACITY (LBS)	REMARKS
HS1	MST126	26 - 10dx1 1/2	-	1515	
	CS16	28-8d	-	1650	
	LSTA30	22-10d	-	1670	
	SA36	22-16d	-	1900	
HS2	MST21	30-16d	-	2760	ATTACH TO 4x OR 2- 2x
	MST48	48 - 10dx1 1/2	-	2850	ATTACH TO 4x OR 2- 2x
	2-MST126	26 - 10dx 1 1/2 EA. STRAP	-	3020	ATTACH TO DOUBLE TJI
	2-MST136	36 - 10dx 1 1/2 EA. STRAP	-	4120	ATTACH TO DOUBLE TJI
HS3	MST48	46-16d	-	4460	ATTACH TO 4x OR 2- 2x
	CMST14	56-10d	-	4470	
HS4	2-MST148	48 - 10dx 1 1/2 EA. STRAP	-	5160	ATTACH TO DOUBLE TJI
	MST60	56-16d	-	5800	ATTACH TO 4x OR 2- 2x
	CMST14	76-10d	-	5860	
HS5	CMST14	88-10d	-	6785	
	HSA55		8-3/4" DIA.	6540	ATTACH TO 4x MEMBER
	2-MST166	66 - 10dx 1 1/2 EA. STRAP	-	7360	ATTACH TO DOUBLE TJI
	HSA66	-	10-3/4" DIA.	8350	ATTACH TO 4x MEMBER
HS6	CMST12	104-10d	-	8060	
	2-MST172	82 - 10dx 1 1/2 EA. STRAP	-	8830	ATTACH TO DOUBLE TJI



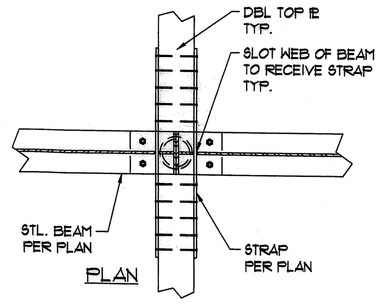
TYPICAL



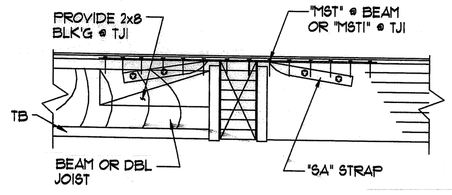
END CONDITION



TYPICAL

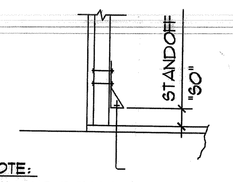


TYP. AT STL. BEAM TO STRAP CONN.

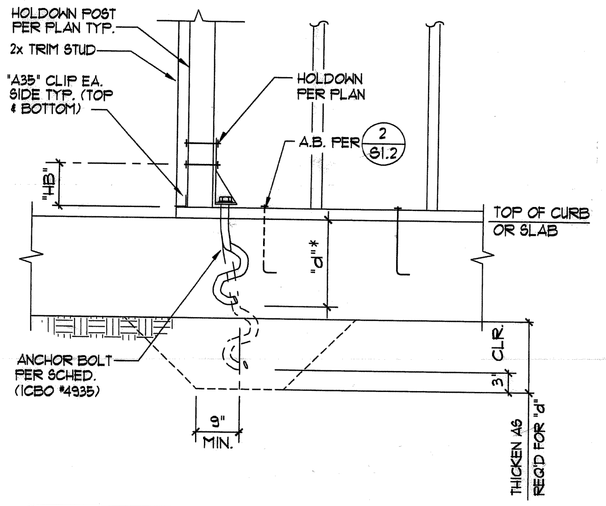


TYPICAL

8



NOTE:
"HDA" HOLDOWN SHOWN FOR "HD" HOLDOWNS, PROVIDE STANDOFF AS REQ'D. PER MANUFACTURER'S DESIGN CRITERIA.



MARK	ANCH.BOLT TYPE	'd'	"HBT"	POST (MIN)	ALT. POST AT 4" NOMINAL WALL	CAPACITY (LBS.)	REMARKS
* HD2A (7)	5/8"ø SSTB20	16	4 1/2"	4x6	-	2715	
* HD5A (7)	5/8"ø SSTB24	20	5 1/2"	4x6	-	4010	
* HD6A (7)	3/4"ø SSTB34	28	6 3/16"	4x6	-	5510	
HD8A	3/4"ø SSTB34	28	6 3/16"	6x6	4x6	7310	

TYPICAL HOLDOWN SCHEDULE

1. *HTT22 WITH 5/8"ø ANCHOR BOLT CAN BE USED AS ALTERNATE OPTION
2. ALL ANCHOR BOLTS AND HARDWARE IN CONTACT WITH FIRE TREATED WOOD MUST BE HOT DIPPED GALVANIZED.

4

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Gary and Mary West
Senior Wellness Center

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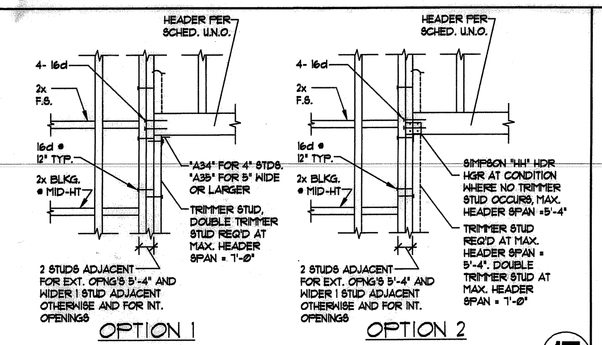
Senior Community Centers

Revision Dates

1	PERMIT SET	7.22.09

Issue Dates

DPR	
Design Development	
Plan Check	6.15.09
Bid Set	
Permit Set	
Construction Set	
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	S1.2



TYP. HEADER FRAMING

17

CONNECTION:	NAILING:
1. Joist to sill or girder, toenail	3-8d
2. Bridging to joist, toenail each end	2-8d
3. 1"x6"(25mm x 152mm) subfloor to each joist, face nail	2-8d
4. Wider than 1"x6"(25mm x 152mm) subfloor to each joist	3-8d
5. 2"(51mm) subfloor to joist or girder, blind & face nail	2-16d
6. Sole plate to joist or blocking,typ. face nail	16d @ 16"(406mm) o.c.
Sole plate to joist or blocking, @ braced wall panels	3-16d per 16"(406mm)
7. Top plate to stud, end nail	2-16d
8. Stud to sole plate	4-8d toenail or 2-16d, end nail
9. Double studs, face nail	16d @ 24"(610mm) o.c.
10. Doubled top plates, typical face nail	16d @ 16"(406mm) o.c.
Doubled top plates,lap splice	8-16d
11. Blocking between joists or rafters to top plate, toe nail	3-8d
12. Rim joist to top plate, toe nail	8d @ 6" (152mm) o.c
13. Top plates, laps and intersections, face nail	2-16d
14. Continuous header, two pieces	16d @ 16" (406mm) o.c. along each edge
15. Ceiling joists to plate, toenail	3-8d
16. Continuous header to stud, toenail	4-8d
17. Ceiling joists, laps over partition, face nail	3-16d
18. Ceiling joists to parallel rafters, face nail	3-16d
19. Rafter to plate, toenail	3-8d
20. 1" (25mm) brace to each stud and plate, face nail	2-8d
21. 1"x8" (25mm x 203mm) sheathing or less to each bearing, face nail	2-8d
22. Wider than 1"x8" (25mm x203mm) sheathing to ea. bearing, face nail	3-8d
23. Built-up corner studs	16d @ 24" (610mm) o.c.
24. Built-up girder and beams	20d @ 32" (813mm) o.c. at top and bott. and stagg. 2-20d at ends and at each splice
25. 2" (51mm) planks	2-16d @ each bearing
26. Wood structural panels and particleboard : Subfloor, roof and wall sheathing (to framing) : (1" = 25.4 mm) 1/2" and less	6d
19/32" - 3/4"	8d or 6d
7/8" - 1"	8d
1 1/8" - 1 1/4"	10d or 8d
Combination subfloor-underlayment (to framing) : (1" = 25.4mm) 3/4" and less	6d
7/8" - 1"	8d
1 1/8" - 1 1/4"	10d or 8d
27. Panel siding (to framing): 1/2" (13mm) or less	6d
5/8" (16mm)	8d
28. Fiberboard sheathing: 1/2" (13mm)	No. 11 ga. 8d
	No. 16 ga.
25/32" (20mm)	No. 11 ga. 8d
	No. 16 ga.
29. Interior panelling 1/4" (6.4mm)	4d
3/8" (9.5mm)	6d

Note:
Diaphragm sheathing nails or other approved sheathing connectors shall be driven so that their head or crown is flush with the surface of the sheathing.

COMMON NAILS NAILING SCHEDULE

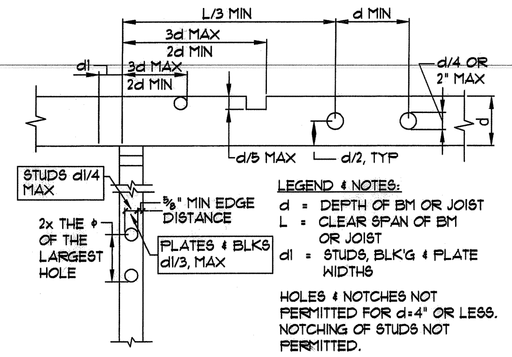
18

BEAM - HEADER SCHEDULE			
ROOF		FLOOR	
SPAN	SIZE	SPAN	SIZE
Ø TO 4'-Ø"	2- 2x4 OR 4x4	Ø TO 4'-Ø"	2- 2x6 OR 4x6
4'-1" TO 6'-Ø"	2- 2x6 OR 4x6	4'-1" TO 6'-Ø"	2- 2x8 OR 4x8
6'-1" TO 8'-Ø"	2- 2x8 OR 4x8	6'-1" TO 8'-Ø"	2- 2x10 OR 4x10
8'-1" TO 10'-Ø"	2- 2x10 OR 4x10	8'-1" TO 10'-Ø"	2- 2x12 OR 4x12
10'-1" TO 12'-Ø"	2- 2x12 OR 4x12	10'-1" TO 12'-Ø"	2- 2x14 OR 4x14

NOTES:
1. REFER TO (n) FOR HEADER FRAMING.
2. THIS SCHEDULE APPLIES UNLESS OTHERWISE NOTED ON DRAWINGS.
3. FOR 2x6 STUD WALLS USE (3) 2x OR 6x SAME DEPTH AS SCHEDULED ABOVE.
4. FOR FLUSH BEAM CONDITION DO NOT USE (2) 2x OR (3) 2x.

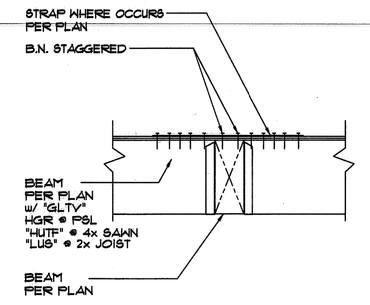
TYP. BEAM HEADER

13



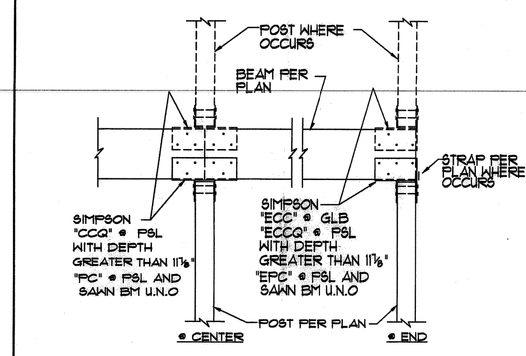
TYP. HOLES & NOTCHES IN 2X JOIST

9



TYP. BEAM TO BEAM CONN.

5



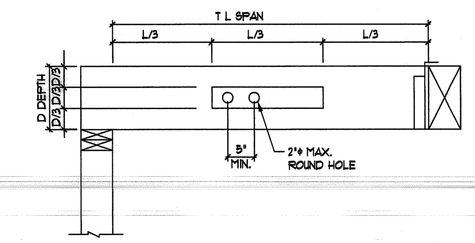
TYP. WOOD BEAM TO WOOD POST CONNECTIONS

1

WOOD BEAM & JOIST CONNECTOR SCHEDULE		
BEAM/JOIST SIZE	SIMPSON CONNECTOR	REMARKS
2X4 thru 2X14 (level)	U series hangers	
2X4 thru 2X14 (sloping)	LSSU series hangers	(2) (4)
4X6 thru 4X12 (level)	HU (MAX) series hangers	
TJI/PRO (level)	ITT series	
TJI/PRO (sloping)	LSSU series	(2) (4)
3 1/2 PSL	EGP series hangers	
5 1/4 PSL	EGP series hangers	
7 PSL	EGP series hangers	

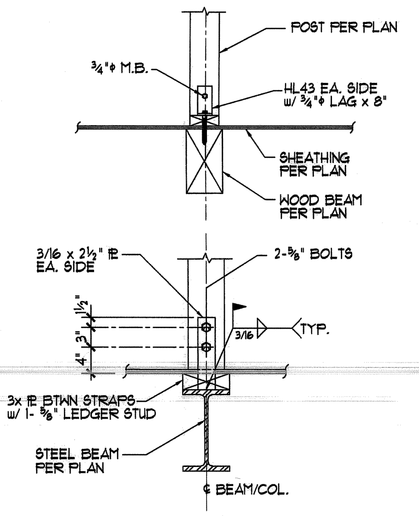
CONNECTOR SCHEDULE NOTES:
1. HANGERS SHOWN ARE TO BE USED UNLESS ANOTHER TYPE OF HANGER IS CALLED FOR ON THE PLANS OR DETAILS.
2. ALL HANGERS MUST BE FULLY NAILED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S DIRECTIONS.
3. HANGERS EXPOSED TO THE EARTH OR WEATHER MUST BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A153.
4. TJI/PRO JOISTS USED WITH LSSU SERIES HANGERS MUST BE PROVIDED W/ WEB STIFFENERS IN ACCORDANCE WITH JST MANUFR.'S RECOMMENDATIONS.
5. DEPTH OF BEAM HANGER (H) MUST BE AT LEAST 90% OF THE DEPTH OF THE BEAM.
6. UNLESS A STEEL BEAM IS PROVIDED WITH A 4X WOOD NAILER TOP FLANGE HANGERS ATTACHED TO THE STEEL BEAM MUST BE WELDED TO THE TOP FLANGE OF THE BEAM U.N.O.

14



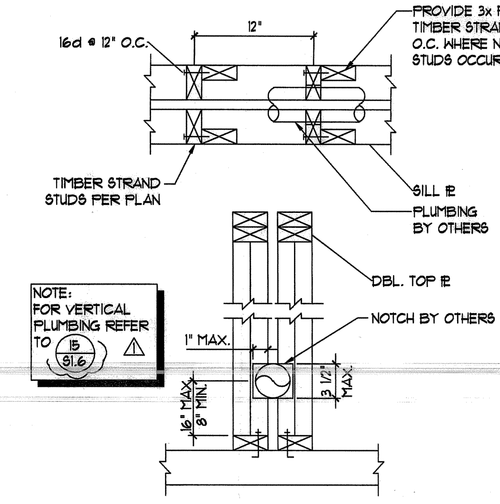
TYPICAL HOLE ON SAWN & PSL BEAM

10



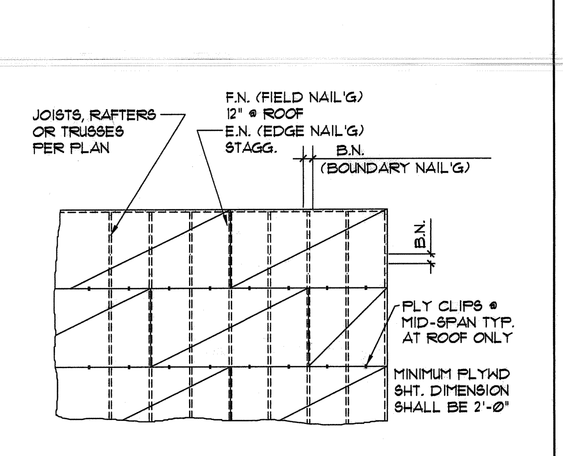
TYP. NON-HOLDOWN POST TO BEAM CONN.

6



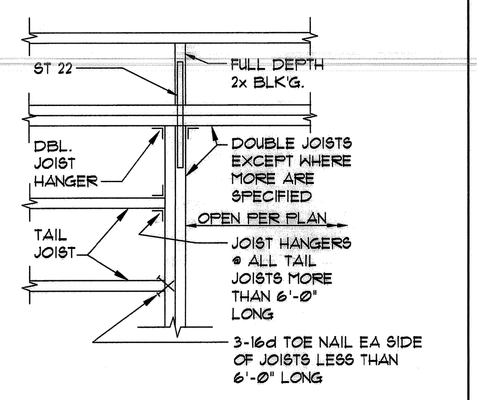
ALT. FRAMING AT PLUMBING

2



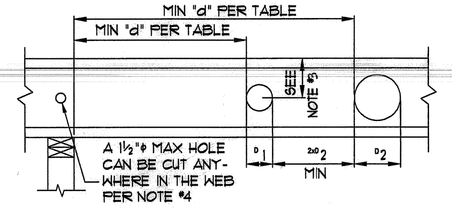
TYP. DIAPHRAGM NAILING

19



TYP. JOIST SUPPORT AT OPENING

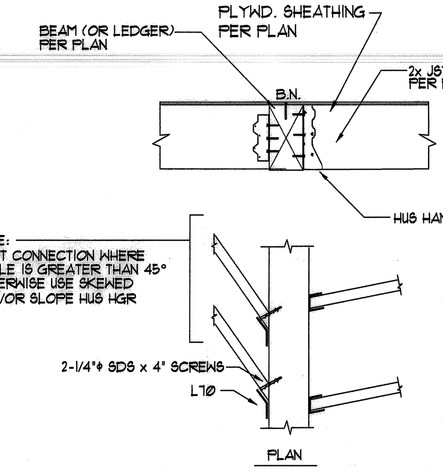
15



TJI JOIST HOLE SCHEDULE				
JOIST DEPTH	JOIST SERIES	"d" SEE NOTE # HOLE DIAMETER 2"	3"	4"
9½"	TJI/25DF	2'-Ø"	3'-Ø"	4'-6"
11⅞"	TJI/35DF	1'-Ø"	1'-Ø"	1'-Ø"
	TJI/25DF	1'-Ø"	1'-Ø"	2'-Ø"
14"	TJI/25DF	1'-Ø"	1'-Ø"	1'-Ø"

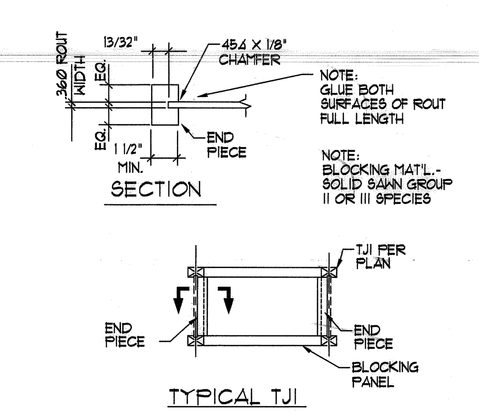
NOTES:
1. MIN. DISTANCE 'd' FROM INSIDE FACE OF ANY SUPPORT TO NEAREST EDGE OF HOLE.
2. IF MORE THAN ONE HOLE IS TO BE CUT IN THE WEB, THE LENGTH OF THE UNCUT WEB BETWEEN HOLES MUST BE TWICE THE LENGTH OF THE LONGEST DIMENSION OF THE LARGEST ADJACENT HOLE.
3. HOLES SHOULD BE LOCATED VERTICALLY AT THE CENTER DEPTH WITHIN THE WEB.
4. TJI JOISTS ARE MANUFACTURED WITH ⅛" PERFORATED 'KNOCKOUTS' IN THE WEB AT APPROXIMATELY 12" ON CENTER ALONG THE LENGTH OF THE JOIST.
5. DO NOT CUT INTO JOIST FLANGES WHEN CUTTING OUT WEB.

11



TYP. SKEWED JST. FRAMING TO BEAM/LEDGER

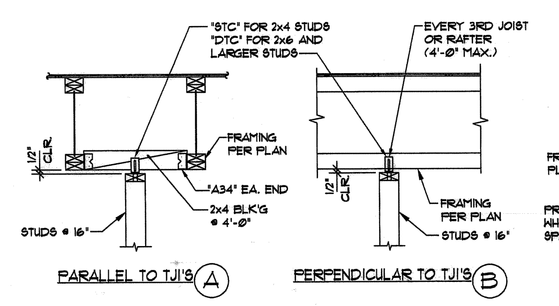
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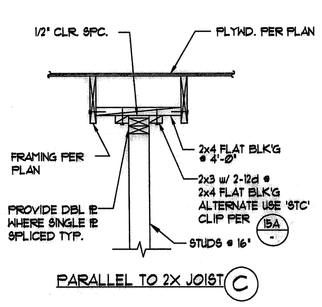


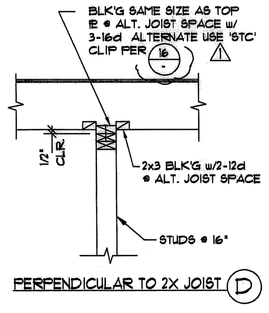
TYPICAL TJI

BLOCKING DETAIL

3

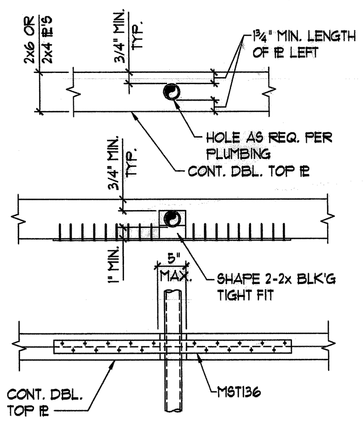






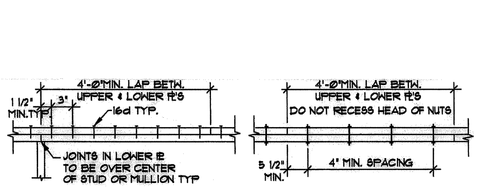
TYP. TOP CONN. @ NON-BEARING NON-SHEAR WALLS

16



TOP PLATE SPLICE @ PENETRATIONS

12



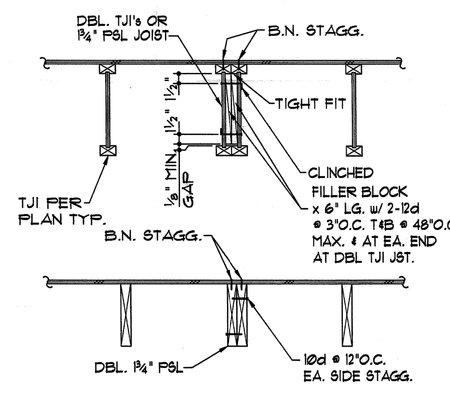
SPL	NAILING	LOAD(lbs)	SPL(3)	NAILING	LOAD(lbs)	SPL	BOLTING	LOAD(lbs)
A	8-16d (4)	1138 (1)	F	20-16d	2846 (2)	K	2-5/8" M.B.	1510
B	10-16d (4)	1423 (1)	G	24-16d	3414 (2)	L	3-5/8" M.B.	2350
C	12-16d (4)	1707 (1)	H	28-16d	3984 (2)	M	2-3/4" M.B.	1889
D	14-16d (4)	1992 (1)	I	32-16d	4552 (2)	N	3-3/4" M.B.	2833
E	16-16d (4)	2276 (1)	J	36-16d	5122 (2)	O	4-3/4" M.B.	3777

NOTE:
1. MIN. SPLICE 16d @ 12"o.c. U.N.O
2. NUMBER OF NAILS OR BOLTS LISTED BELOW SHALL BE USED EACH SIDE OF EACH UPPER & LOWER IE JOINT

(1)REQUIRES 9 GRADE 2X4 TOP IE'S MIN. OR STRUCT. 1 INDEX (40/20) PLYWOOD
(2)REQUIRES 9 GRADE 2X4 TOP PER MIN.
(3)REQUIRES GREATER THAN 4'-8" LAP SPLICES
(4)REQUIRES 8d NAILS IN LIEU OF 16d NAILS

TYP. DOUBLE TOP PLATE SPLICE

8



TYP. DOUBLE JOISTS ATTACHMENT
(NOTED AS "TB" ON PLANS)

4

FLC FLORES LUND CONSULTANTS
7220 TRADE STREET, SUITE 120
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Gary and Mary West Senior Wellness Center

1525 Fourth Avenue
Senior Community Center

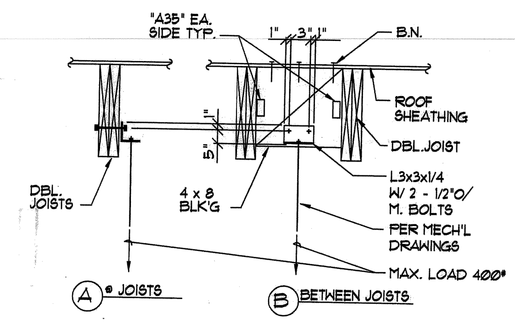
Senior Community Center

Revision Dates
- PERMIT SET 7.22.09

Issue Dates
- DPR
- Design Development
- Plan Check 6.15.09
- Bid Set
- Permit Set
- Construction Set

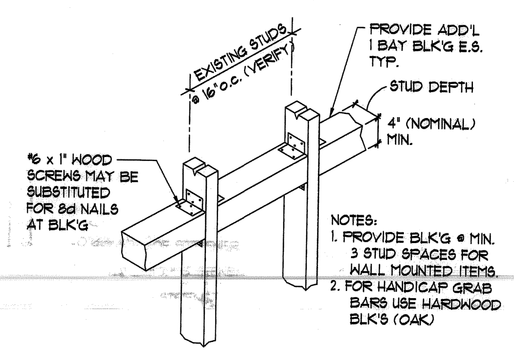
Drawing Date 6-8-09
Check By RHF
Drawn By KBM
Scale AS SHOWN
Job Number FLC09023
Sheet Number

S1.3



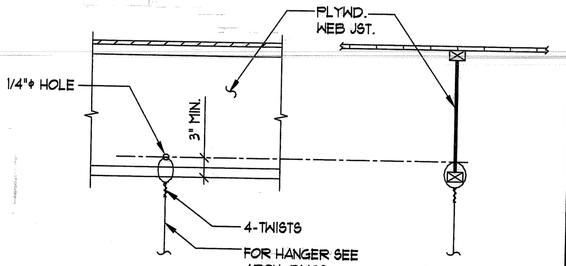
TYPICAL EQUIPMENT HANGERS

17



CASE WORK, MECH'L, ELECT., ETC. SUPPORT

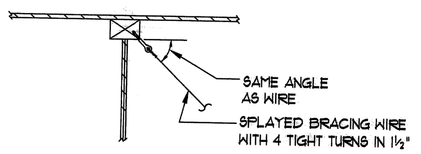
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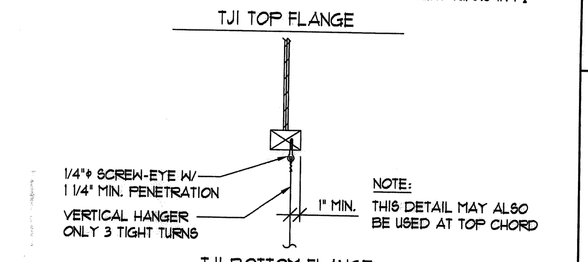
ELEVATION

SECTION

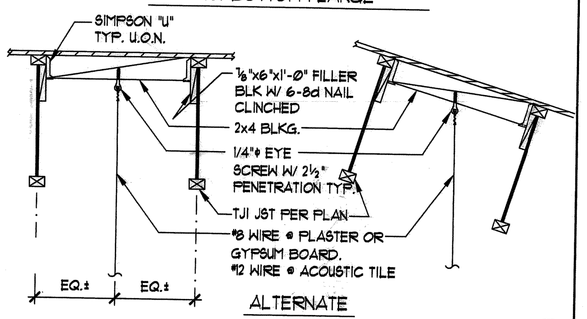
NOTE: DO NOT INSERT SCREW-EYES IN SIDE OF MICRO-LAM FLANGE.



TJI TOP FLANGE



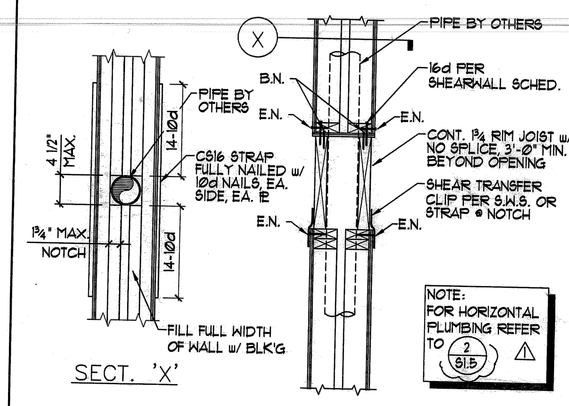
TJI BOTTOM FLANGE



ALTERNATE

TYPICAL HANGER DETAIL FOR SUSPENDED ACOUSTIC CEILING

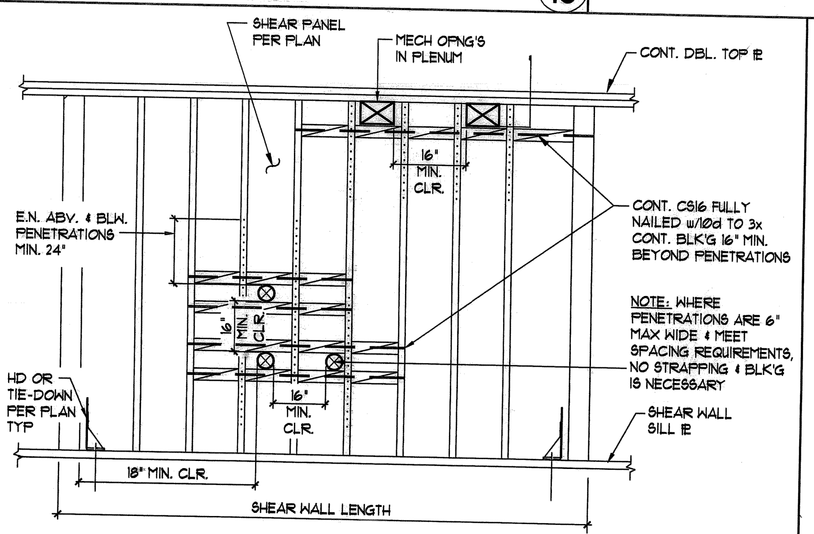
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SECT. 'X'

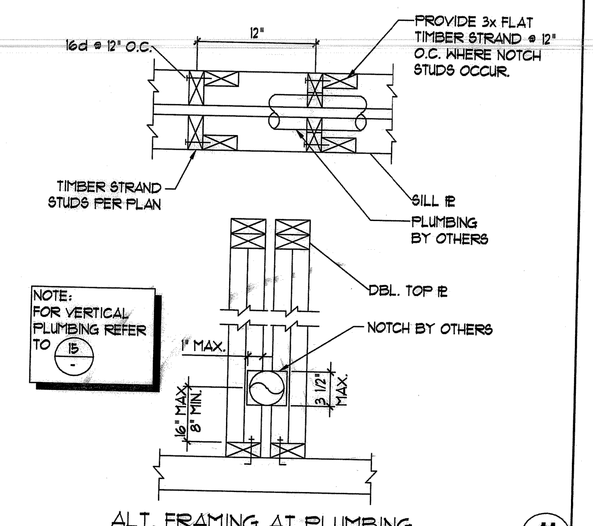
STRAPPING AT PIPING INTERESECTION OF SHEAR WALL TOP E'S

15



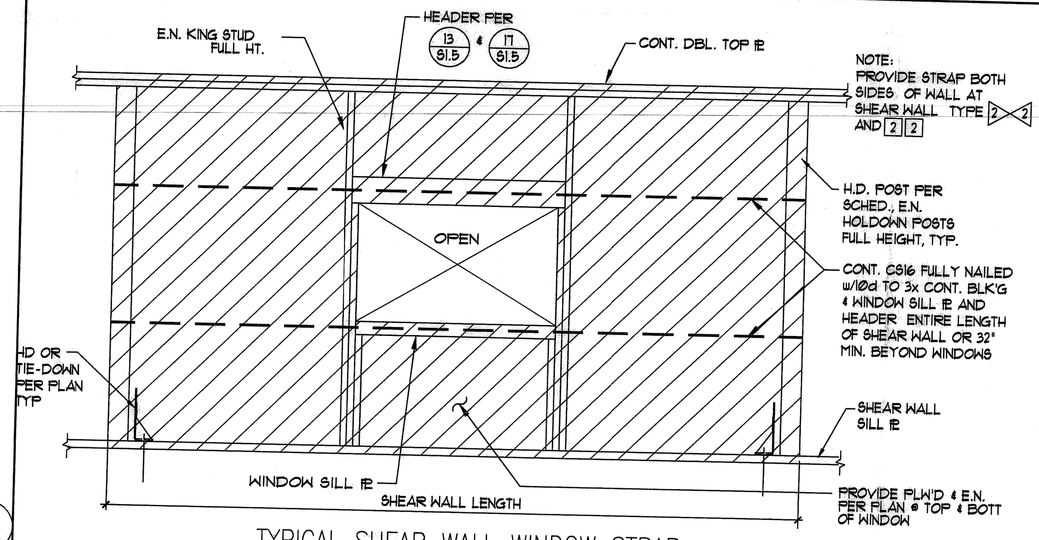
TYP. FRAMING FOR SMALL PENETRATIONS IN SHEAR PANELS

16



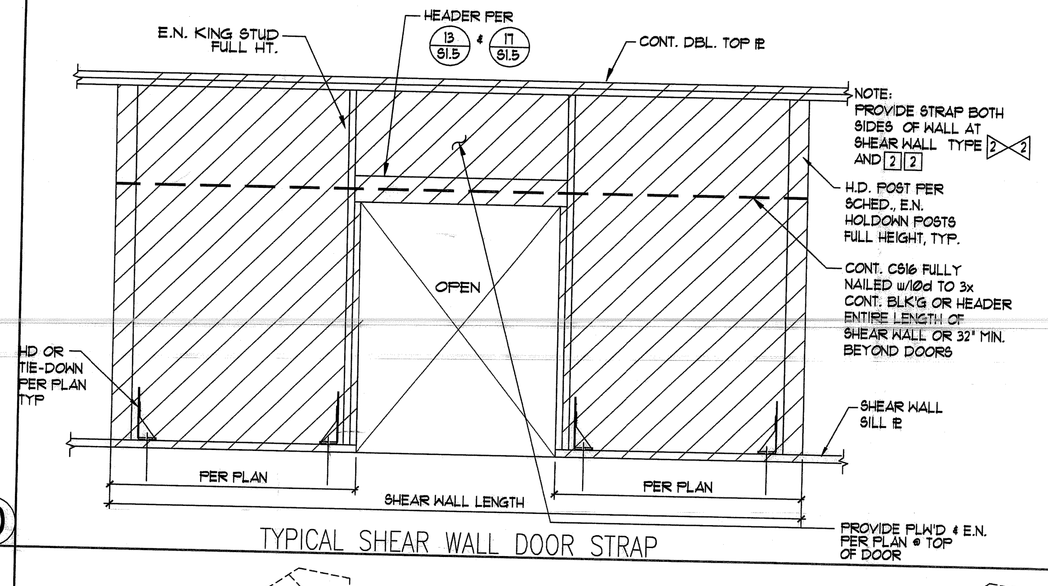
ALT. FRAMING AT PLUMBING

11



TYPICAL SHEAR WALL WINDOW STRAP

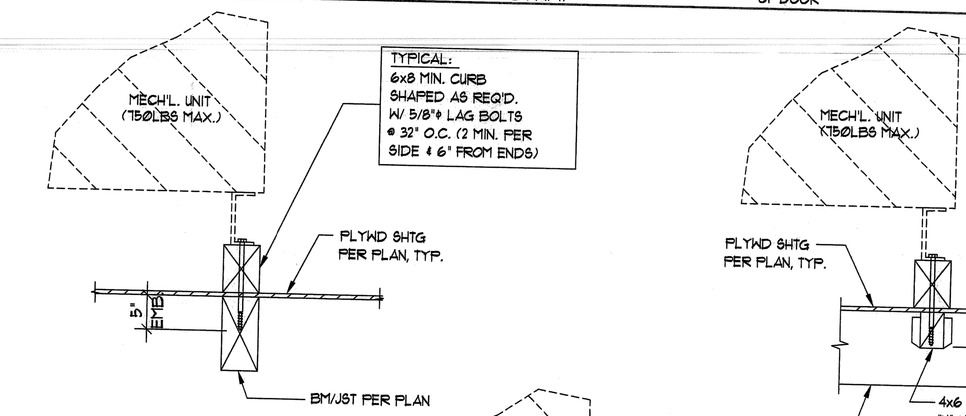
9



TYPICAL SHEAR WALL DOOR STRAP

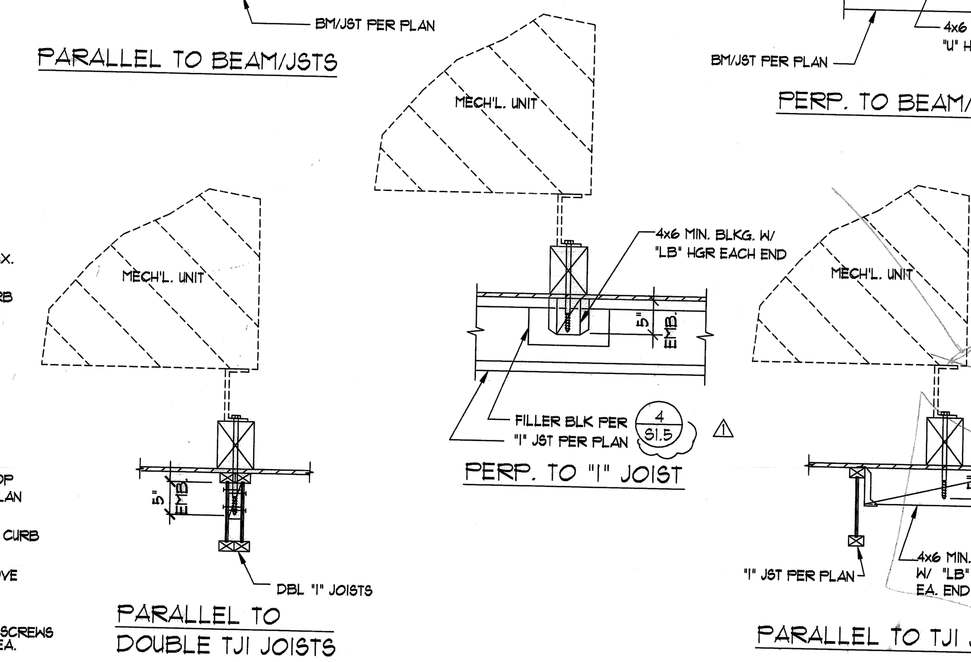
10

2



PARALLEL TO BEAM/JSTS

PERP. TO BEAM/JOIST

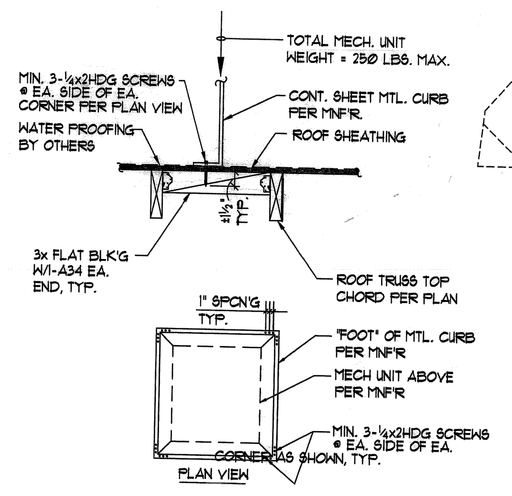


PERP. TO "I" JOIST

PARALLEL TO DOUBLE TJI JOISTS

PARALLEL TO TJI JOISTS

4



ROOF TRUSSES

TYP. MECH. ANCHORAGE TO WOOD FRAMING

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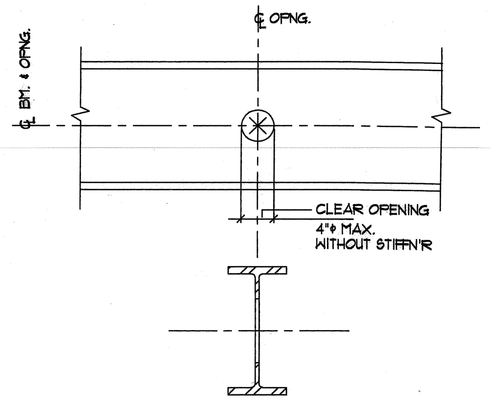
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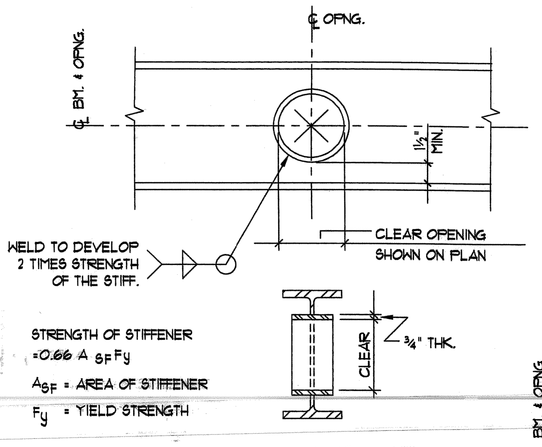
Senior Community Centers

Revision Dates	
1	PERMIT SET 7.22.09
2	
3	
4	
5	
Issue Dates	
DPR	
Design Development	
Plan Check	6.15.09
Bid Set	
Permit Set	
Construction Set	
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
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Sheet Number	S1.4



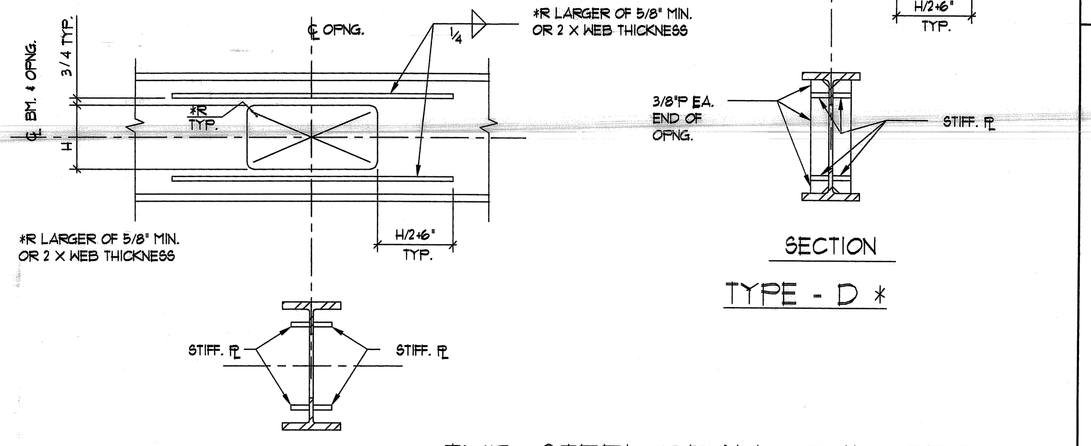
SECTION

TYPE - A *



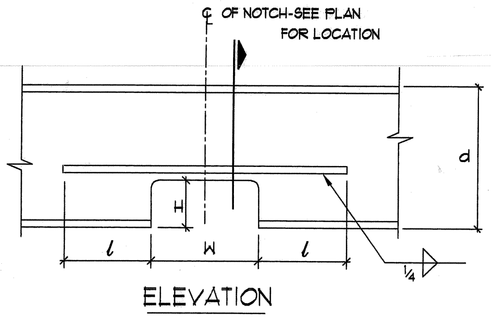
SECTION

TYPE - B *

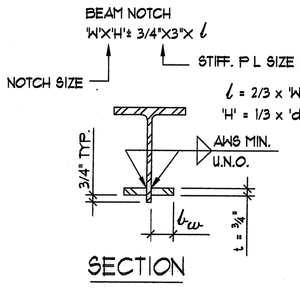


SECTION

TYPE - C *



ELEVATION



SECTION

TYP. BEAM NOTCH DETAIL *

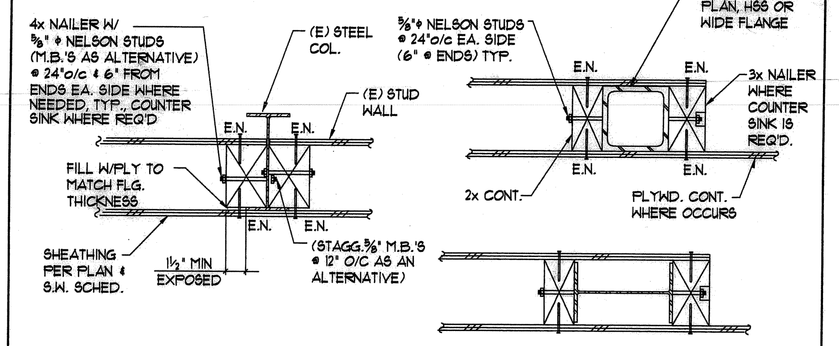
SECTION

TYPE - D *

TYP. STEEL BEAM OPN'G REQ'MTS

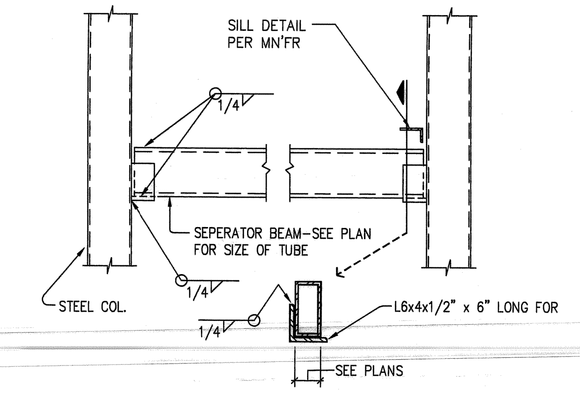
* NOTE: EDGE OF ANY OPENING ABV. TO BE NO CLOSER THAN 2 x 'd' FROM ℄ OF BEAM SUPPORT COL.

15



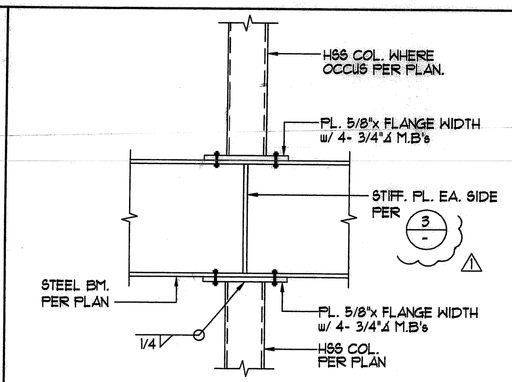
TYP. COL. / NAILER DETAILS

9

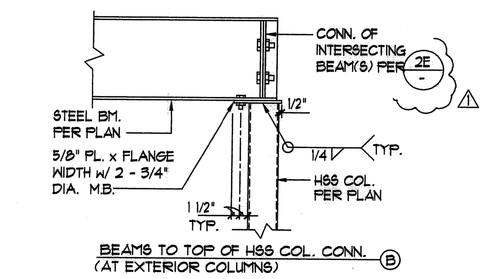


TYP. ELEVATOR SHAFT SEPARATOR BEAM

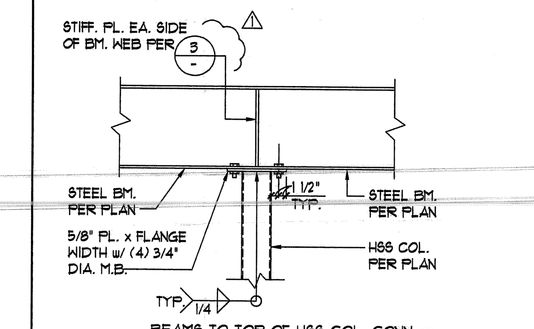
10



BEAM TO COLUMN BEAM TO BEAM
(CONTINUOUS BEAMS AT EXTERIOR COLUMNS) A

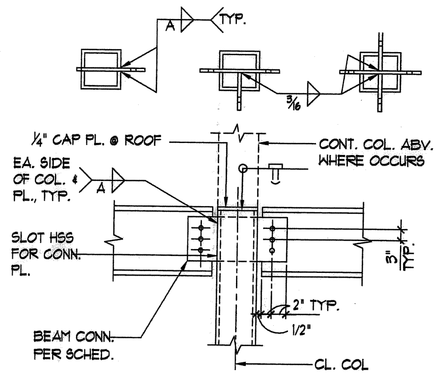


BEAMS TO TOP OF HSS COL. CONN.
(AT EXTERIOR COLUMNS) B

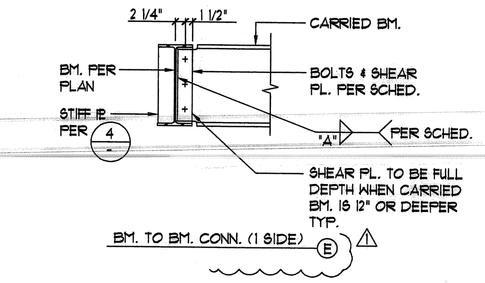


BEAMS TO TOP OF HSS COL. CONN.
(CONTINUOUS BEAMS AT EXTERIOR COLUMNS) C

CONN. BOLT SCHEDULE						
BEAM SIZE	SHEAR PLATE THICKNESS	BOLTS			WELD "A"	REMARKS
		No.	DIA.	TYPE		
W8 x	3/8"	2	¾"	A325-SC	3/16"	
W10 x	3/8"	2	¾"	A325-SC	3/16"	
W12 x	3/8"	3	¾"	A325-SC	3/16"	
W14 x	3/8"	4	¾"	A325-SC	1/4"	
W16 x	3/8"	4	¾"	A325-SC	1/4"	

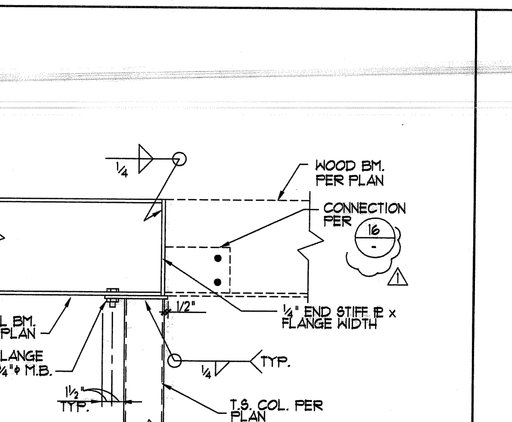


BEAMS TO TOP OF HSS COL. CONN.
(DISCONTINUOUS BEAMS) D

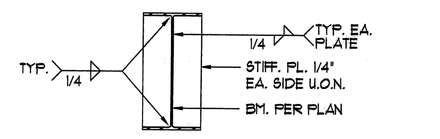


BM. TO BM. CONN. (1 SIDE) E

2

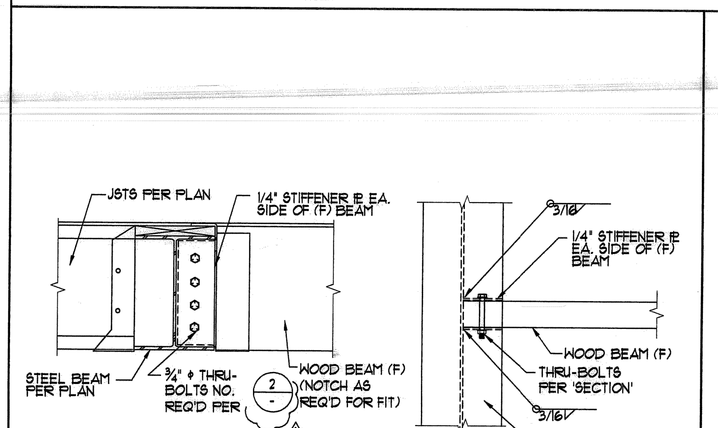


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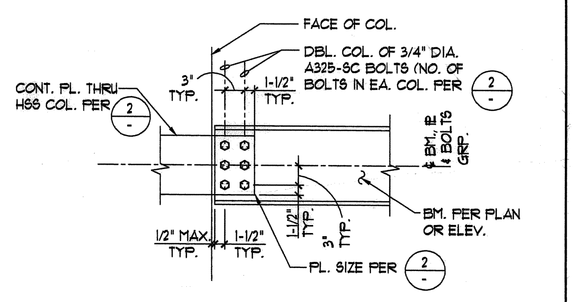
TYP. STIFFENER PLATES

3



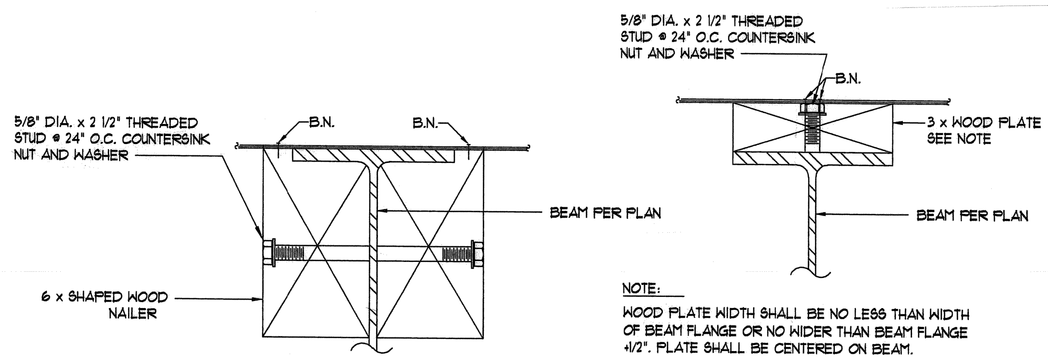
WOOD BM TO STEEL BM
"BUCKET" HANGER

11



NOTES:
1. IF THERE IS A BM. ON LEFT SIDE OF COL. THEN CONN. SHOWN APPLY FOR BOTH SIDES.
2. DIAG. BRACE CONN. WHERE OCCURS NOT SHOWN FOR CLARITY.

12

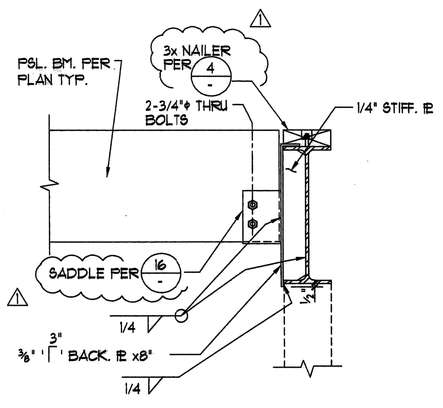


ALTERNATE

NOTE:
WOOD PLATE WIDTH SHALL BE NO LESS THAN WIDTH OF BEAM FLANGE OR NO WIDER THAN BEAM FLANGE +1/2". PLATE SHALL BE CENTERED ON BEAM.

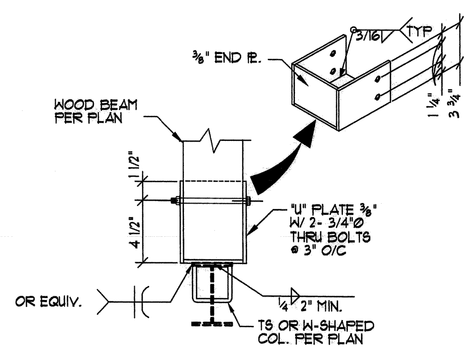
BOLTED WOOD PLATE ON STL. BM.

4



WOOD BM TO STEEL BM
"BUCKET" HANGER (ALT.)

20



WOOD BEAM TO STEEL COLUMN

16

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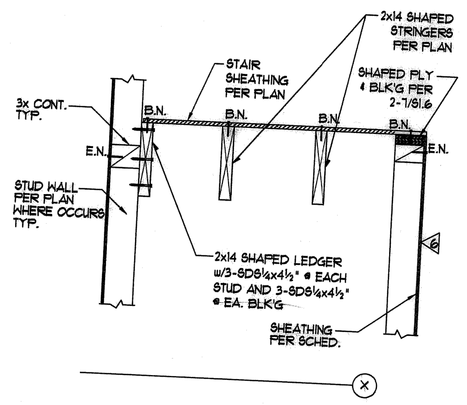
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Senior Community Center

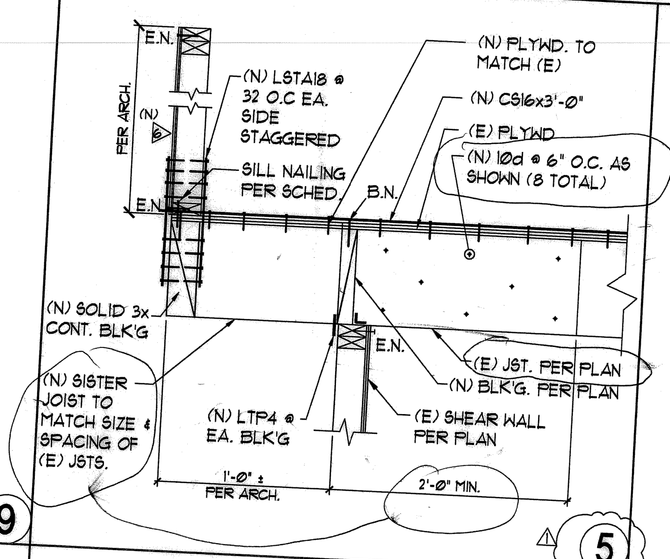
Senior Community Centers

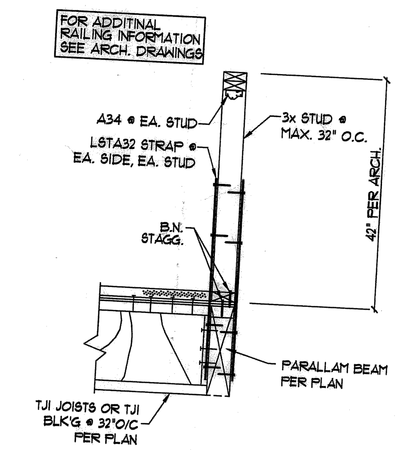
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1	PERMIT SET 7.22.09
2	
3	
4	
5	

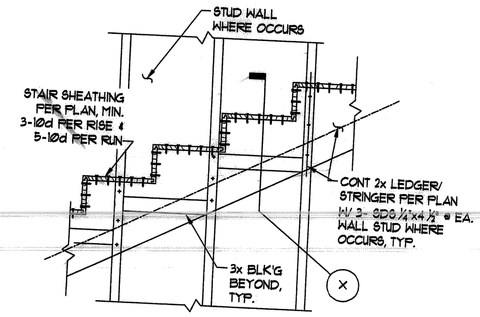
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DPR	.
Design Development	.
Plan Check	6.15.09
Bid Set	.
Permit Set	.
Construction Set	.
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	S1.5

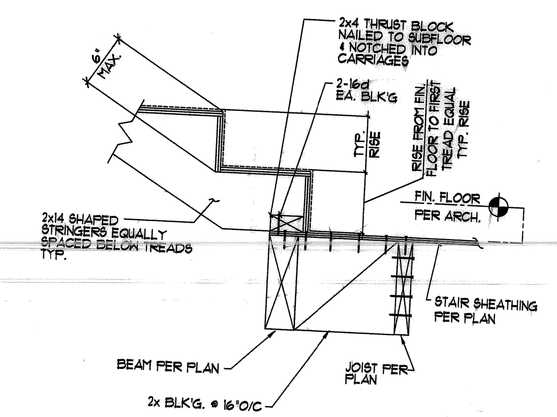
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7220 TRADE STREET, SUITE 120
SAN DIEGO, CALIFORNIA 92121-2325
TEL (858) 566-0626
FAX (858) 566-0627

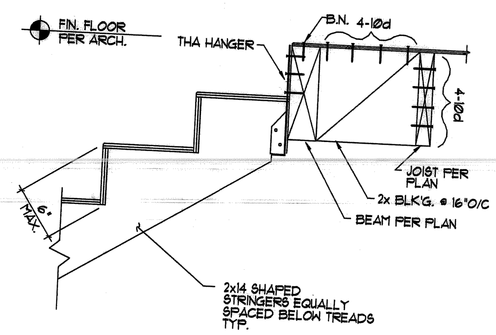












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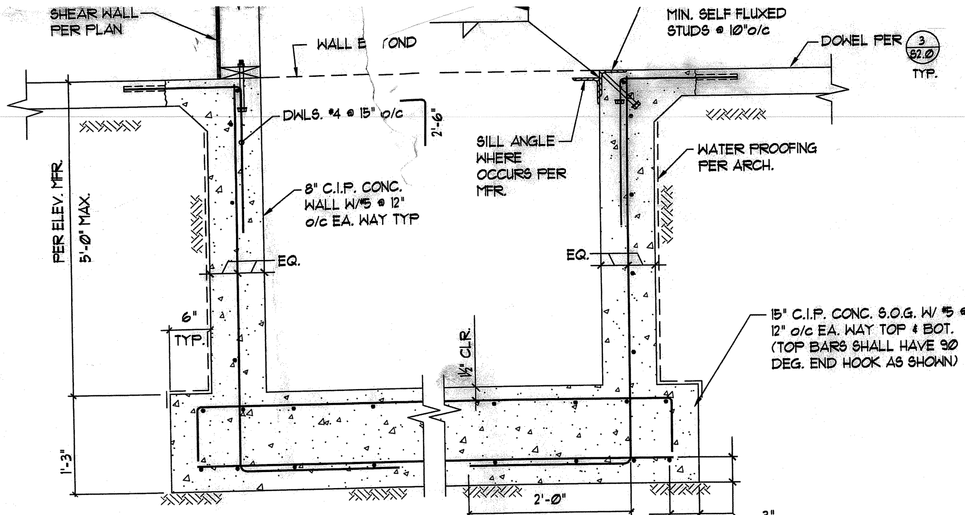
1525 Fourth Avenue
Senior Community Center

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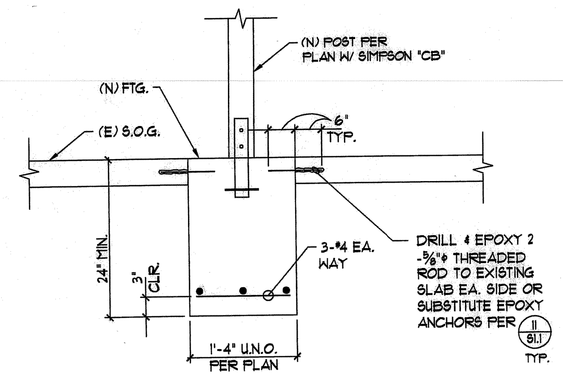
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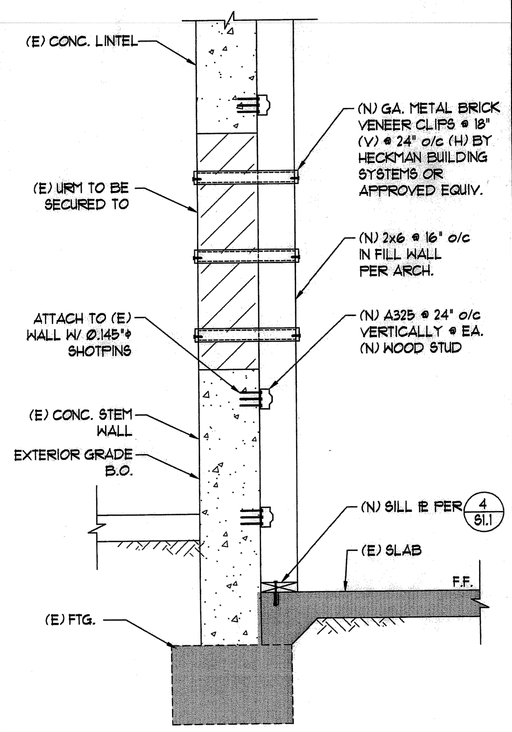
ELEVATOR PIT

13

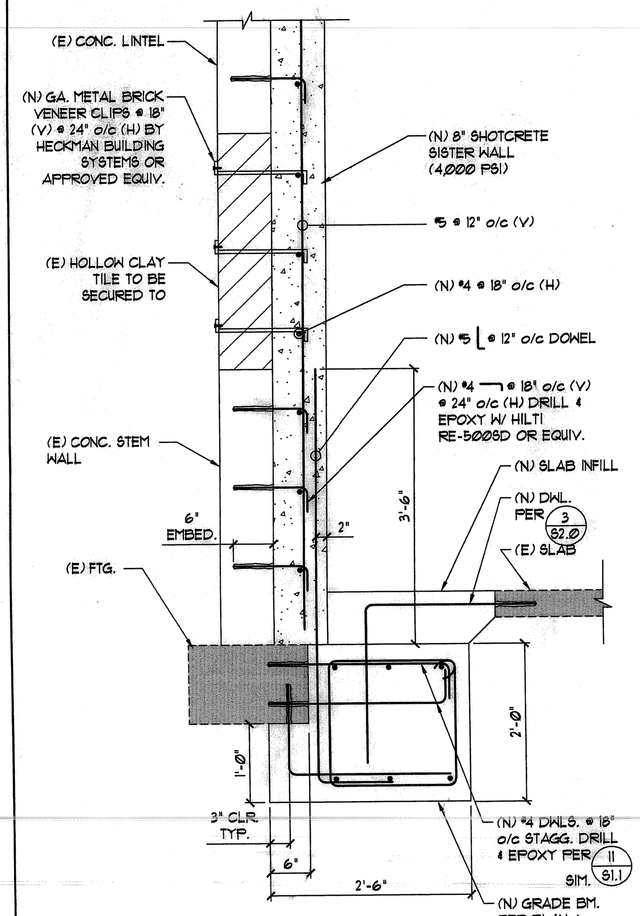


(N) WOOD POST FOOTING

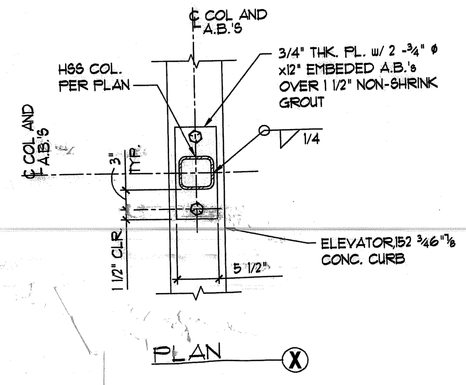
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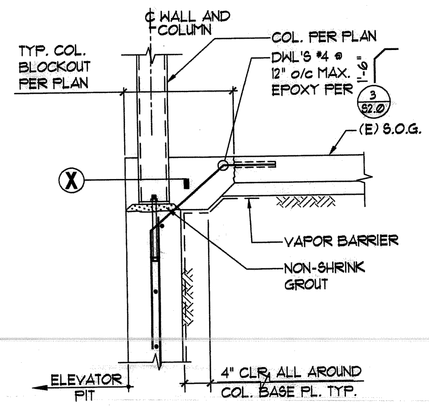
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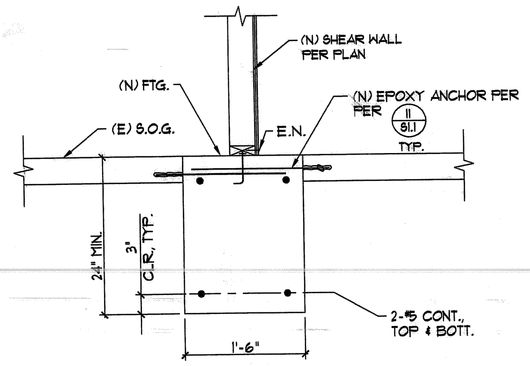


PLAN X



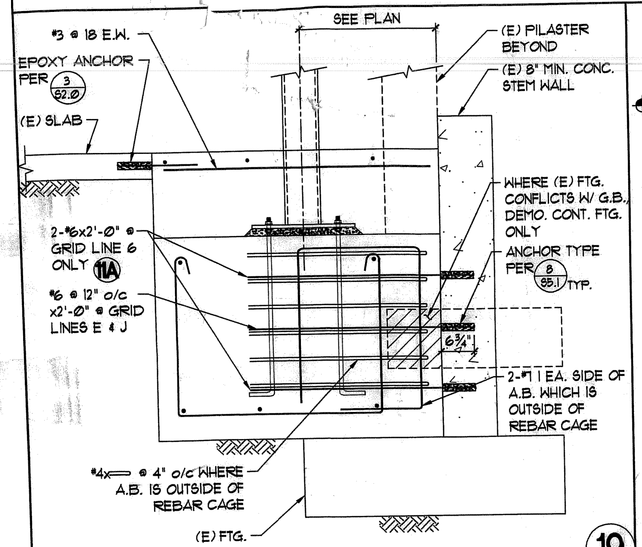
SECTION

14



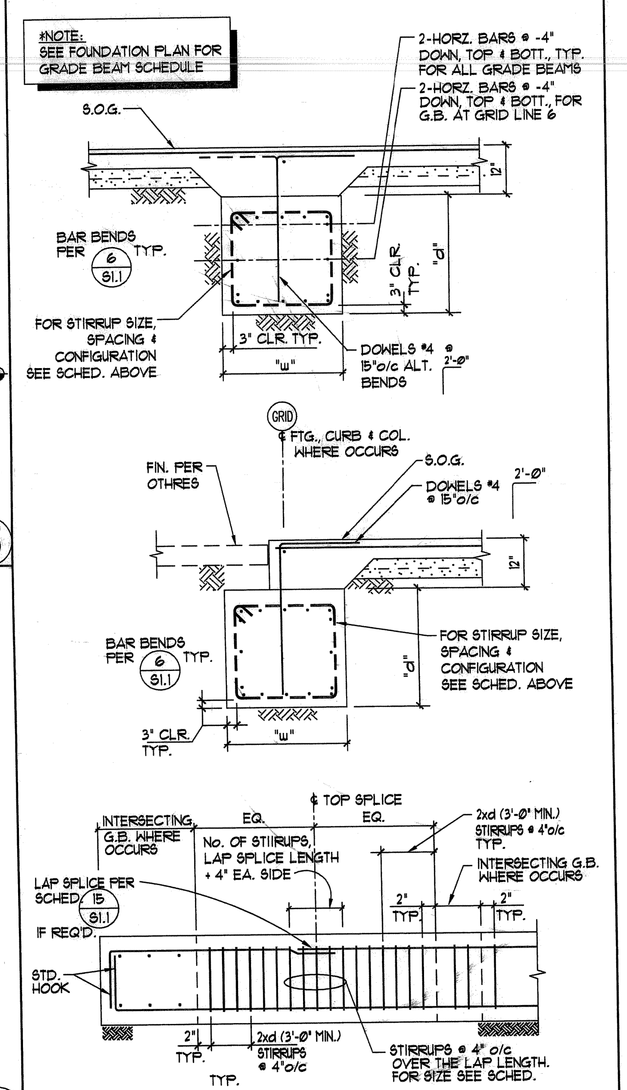
CONTINUOUS FOOTING BLW. (N) SHEAR WALL ON G.L.'S D & G

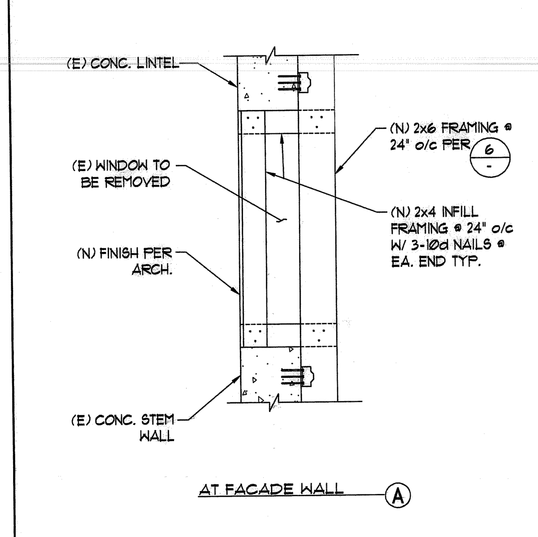
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19

15



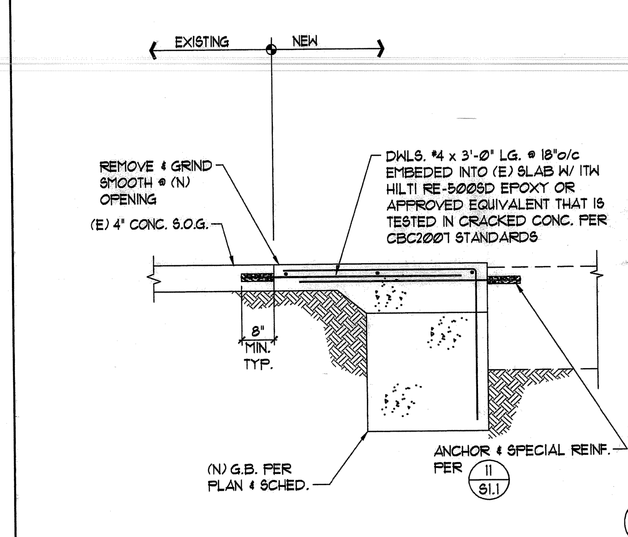


AT FACADE WALL A

AT SHOTCRETE WALL

WINDOW INFILL FRAMING

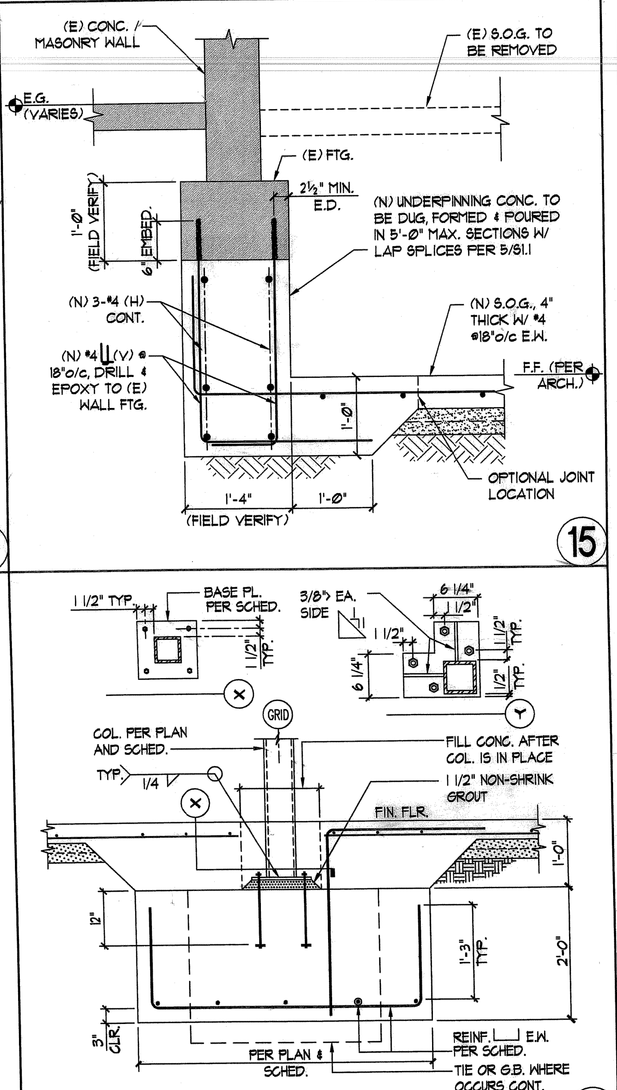
8



3

NOTE: FOR GRADE BEAM SIZE & REINF. SEE PLAN & DETAILS

20



TYPICAL SPREAD FOOTING

16

TYPICAL MIN. DOWELING TO EXISTING STRUCTURE

4

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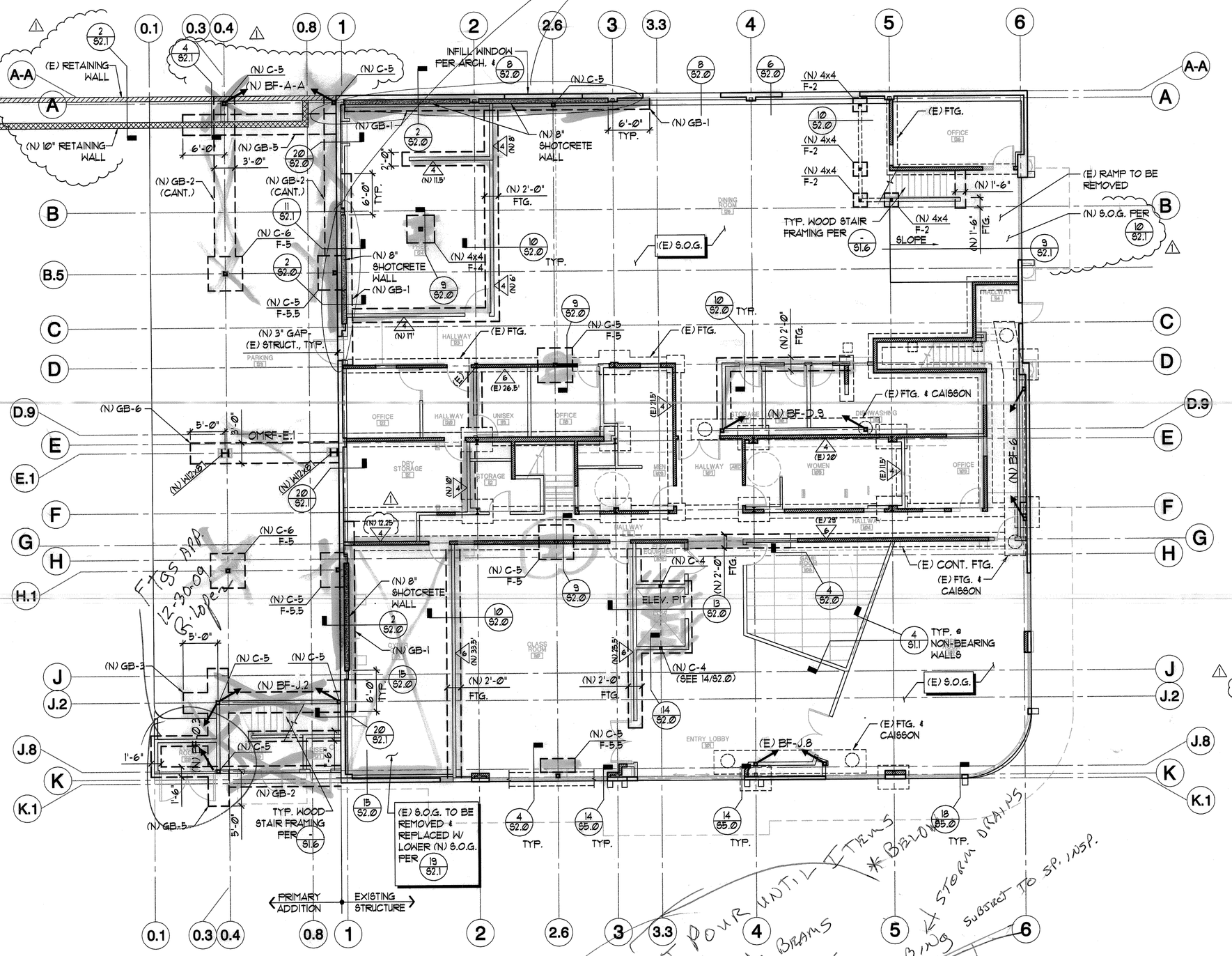
Senior Community Centers

Revision Dates	
1	PERMIT SET 7.22.09
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DPR	
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Plan Check	6.15.09
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Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	S2.0

= Ftgs APP.
12-4-09
R.Lopez S.T.S.I.

Ftgs S.T.S.I. Fwing 8-13-09



= FTgs APP.
12-3-09
R.Lopez
S.T.S.I.

= FOOTINGS STSI REYNOLDS 2.25.09

= FND. WALL REBAR
10-1-09 R.Lopez S.T.S.I.
ALSO PENDING SOILS COMPLIANCE

*DO NOT POUR UNTIL ITEMS * BELOW = FTgs/ GRADE BEAMS *PENDING SOILS AND *PLUMBING & STORM DRAINS SUBJECT TO SP. INSP. S.T.S.I.

= FND WALLS SHOTCRETE S.T.S.I. 10-14-09 R.Lopez

= FTgs APPROVED
10-1-09 R.Lopez
S.T.S.I.
ALSO PENDING SOILS COMPLIANCE

ALL FOOTINGS AND SLAB EXCEPT RETAIN WALLS & TRASH ENCLOSURE.
1-11-10 R.Lopez

FOUNDATION PLAN

SCALE: 1/8"=1'-0"

NORTH

FOUNDATION NOTES:

1. REFER TO SHEETS S1.Ø TO S1.5 FOR GENERAL NOTES & STANDARD DETAILS. THESE NOTES & DETAILS SHALL BE USED WHERE APPLICABLE WHETHER SPECIFICALLY REFERENCED OR NOT.
2. REFER TO ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
   A. ALL DIMENSIONS NOT SHOWN.
   B. ALL OPENINGS NOT SHOWN.
   C. ALL NON-BEARING WALL NOT SHOWN.
3. BF-Ø.3 — INDICATES NEW STEEL BRACE FRAME ABOVE PER PLAN & ELEVATIONS ON S4.Ø.
   OMRF-E.1 — INDICATES NEW STEEL BRACE FRAME ABOVE PER PLAN & ELEVATIONS ON S4.Ø.
4. GB-1 — INDICATES GRADE BEAM MARK PER SCHEDULE
5. INDICATES 8" CMU WALL
6. INDICATES 10" CMU WALL
7. INDICATES EXISTING CONC. STEM WALL
8. ZZZZZZ — HEAVY HATCHED WALL LINES INDICATE EXISTING WALLS
   DASHED WALL LINES INDICATE WALL BELOW
   SOLID WALL LINES INDICATE WALL ABV.
9. INDICATES EXISTING BRICK PILASTER
10. INDICATES CONC. CAISSON PER PLAN.
11. HDU4 — INDICATES HOLDDOWN PER SCHED. SEE (8 / S1.2)
12. 4 / 3' — INDICATES SHEAR WALL MARK FROM THIS LEVEL TO LEVEL ABOVE PER SHEAR WALL SCHEDULE ØN 2/S1.2. PROVIDE NON-SHEAR PLYWOOD ADJACENT TO SHEAR PANELS IN ORDER TO PROVIDE A FLUSH FINISH.
    INDICATES SHEAR WALL PANEL APPROX. MIN. LENGTH IF NOT SHOWN, THEN PROVIDE PLYWOOD ON ENTIRE FACE.
13. (E) INDICATES EXISTING MEMBER
    (N) INDICATES NEW MEMBER
14. REFER TO SHEETS S1.1 FOR ATTACHMENT OF SHEAR WALLS & HOLDOWNS TO (E) SLAB & FOOTINGS.
15. C- INDICATES COL. SIZE PER SCHED. (2 / S2.Ø)
    F- INDICATES FOOTING MARK PER SCHED. (2 / S2.Ø)

FOOTING SCHEDULE

MARK	SIZE	"t"	REINFORCING STEEL	REMARKS
F2	2'-Ø" x 2'-Ø"	24"	3-#4 ☐ EA. WAY	
F4	4'-Ø" x 4'-Ø"	24"	5-#5 ☐ EA. WAY	
F5	5'-Ø" x 5'-Ø"	24"	6-#7 ☐ EA. WAY	
F5.5	5'-Ø" x 5'-Ø"	24"	6-#7 ☐ EA. WAY	5FT FTG. CENTERED BLW. COL., WHERE (E) FTG. INTERRUPTS DOWEL PER (4 / S2.Ø)

COLUMN SCHEDULE

MARK	SIZE	TYP. BASE PLATE AND A.B's U.N.O.	REMARKS
C4	HSS 4x4x1/4	PL. 3/4"x11"x11" w/ 4-3/4"ø 1" EMBED. A.B.'s	SEE 14/S2.Ø FOR ELEV. GUARD RAIL SUPPORT COL'S.
C5	HSS 5x5x3/8	PL. 3/4"x12"x12" w/ 4-3/4"ø 1" EMBED. A.B.'s	
C6	HSS 6x6x5/8	PL. 1-1/4"x14"x14" w/ 4-7/8"ø 1" EMBED. A.B.'s	

NOTE:
COLUMN & BASE PLATES SHOWN FOR GRAVITY COL'S ONLY. FOR COLUMN BASE PLATES THAT ARE PART OF THE BRACED FRAMED SYSTEM, SEE SHEET S4.1.

GRADE BEAM SCHEDULE

MARK	"d"	"w"	TOP REBAR	BOTT. REBAR	FACE REBAR	STIRRUPS	REMARKS
GB-1	24"	24"	5-#6	4-#6		#4 @ 16"o/c	TIES ☐ WHERE POSSIBLE EXTEND G.B. 6'-Ø" BEYOND END OF SHOTCRETE WALL
GB-2	30"	36"	6-#7	6-#7	1-#5	#4 @ 9"o/c	TIES ☐
GB-3	36"	48"	7-#7	10-#7	2-#5	#4 @ 9"o/c	TIES ☐
GB-4	36"	48"	8-#6	9-#6	2-#5	#4 @ 9"o/c	TIES ☐
GB-5	36"	48"	12-#8	12-#8	2-#5	#4 @ 9"o/c	TIES ☐ EXTEND G.B. 5'-Ø" MIN. BEYOND B.F. COL'S.
GB-6	36"	60"	9-#8	11-#8	1-#5	#4 @ 9"o/c	TIES ☐

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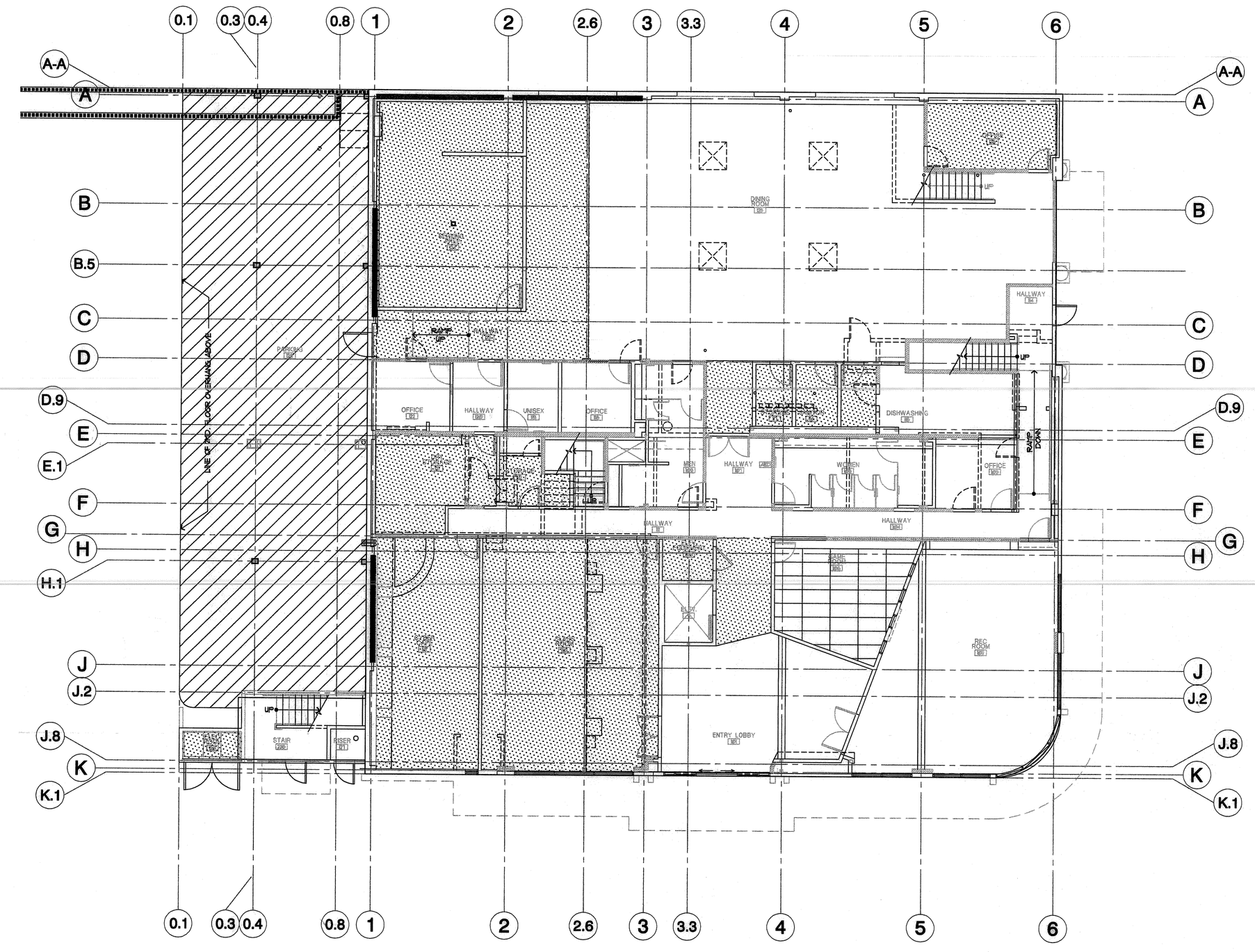
Gary and Mary West Senior Wellness Center
1525 Fourth Avenue
Senior Community Center

Senior Community Centers

Revision Dates	
1	PERMIT SET 7.22.09
2	
3	
4	
5	

Issue Dates	
DPR	
Design Development	
Plan Check	6.15.09
Bid Set	
Permit Set	
Construction Set	
Drawing Date	6-8-09
Check By	RHF
Drawn By	KBM
Scale	AS SHOWN
Job Number	FLC09023
Sheet Number	S3.0

2ND SUBMITTAL



DEMO PLAN

SCALE: 1/8"=1'-0"

NORTH

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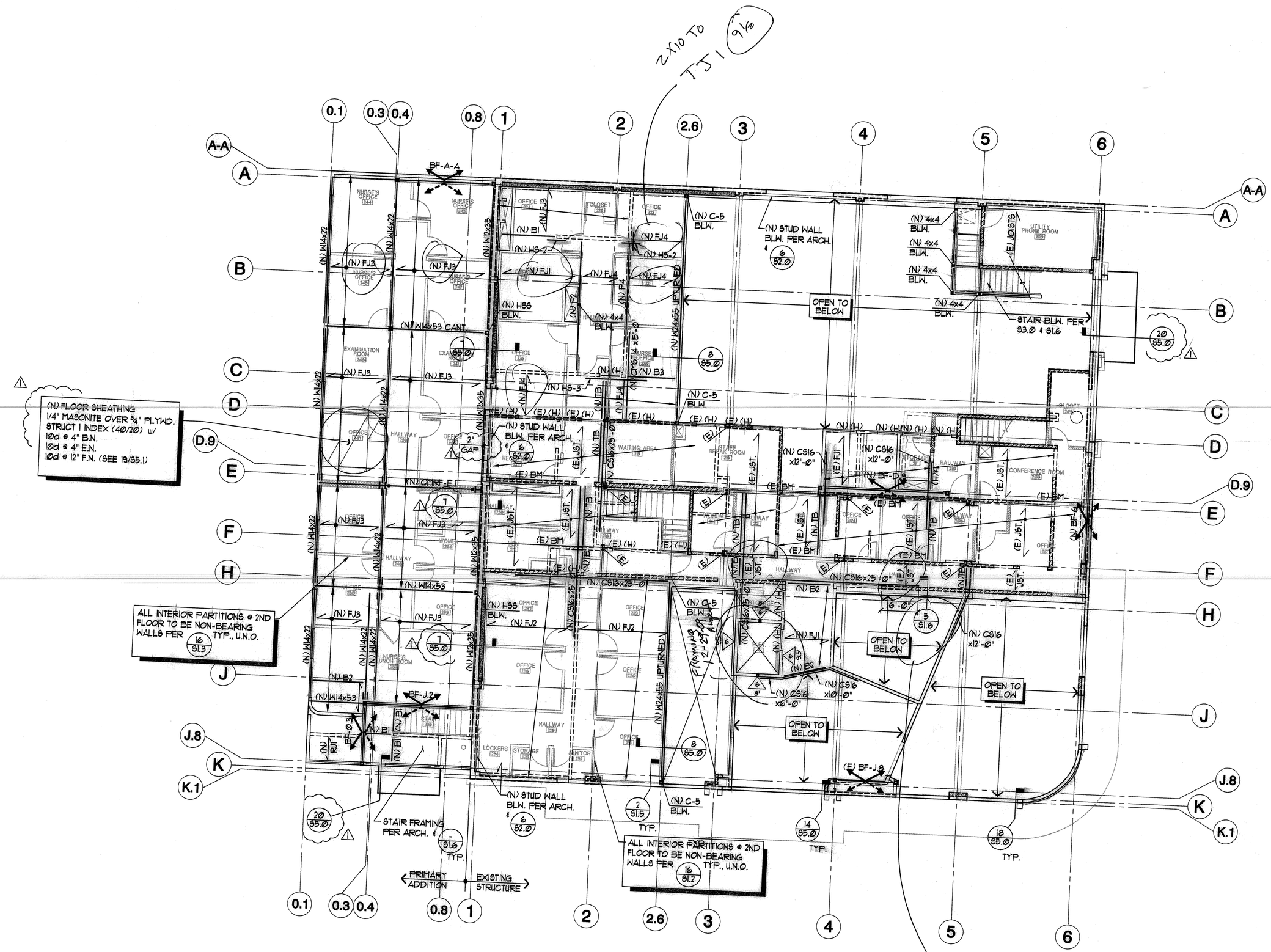
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MEZZANINE FLOOR FRAMING PLAN

SCALE: 1/8"=1'-0"

NORTH

ROUGH FRAMING IS APPROVED EXCEPT THE FOLLOWING @ GRIDLINE 1-6 AND A-K

*CORRECTIONS

(1) SOUTH EAST STAIRWAY

*(2) MEZZANINE 2ND FLOOR STRUCTURAL STEEL BEAMS TO LOW IN HALLWAYS. CITY STRUCTURAL PLAN CHECK APPROVED FIX REQUIRED FOR HEAD ROOM CLEARENCE, PER PREVIOUS REQUEST.

EXISTING JOIST DO NOT BEAR ON EXISTING OR N.

* CORRECTION

FLOOR FRAMING NOTES:

1. REFER TO SHEETS S1.0 TO S1.6 FOR GENERAL NOTES & STANDARD DETAILS. THESE NOTES & DETAILS SHALL BE USED WHERE APPLICABLE WHETHER SPECIFICALLY REFERENCED OR NOT.
2. REFER TO ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
   A. ALL DIMENSIONS NOT SHOWN.
   B. ALL OPENINGS NOT SHOWN.
   C. ALL NON-BEARING WALL NOT SHOWN.
3. BF-0.3 / BF-0.3 — INDICATES NEW STEEL BRACE FRAME BELOW/ABOVE PER PLAN & ELEVATIONS ON S4.0.
   OMRF-E.1 — INDICATES NEW STEEL BRACE FRAME ABOVE PER PLAN & ELEVATIONS ON S4.0.
4. (E) INDICATES EXISTING CONSTRUCTION
   (N) INDICATES NEW CONSTRUCTION
5. INDICATES STEEL BM DRAG CONNECTION PER 12/S1.5 TYP.
6. HEAVY HATCHED WALL LINES INDICATE EXISTING WALLS
   DASHED WALL LINES INDICATE WALL BELOW
   SOLID WALL LINES INDICATE WALL ABV.
7. REFER TO 13/S1.3 FOR BEAM/HEADER SIZE, UNO
8. (N) HS-3 INDICATES HORIZONTAL STRAP PER 8/S1.2 PROVIDE HS-2, U.N.O.
9. (N) CS16 INDICATES HORIZONTAL STRAPPING FROM (N) TO (N) OR (E) FRAMING SIM. TO 8/S1.2
10. SPLICE DOUBLE TOP PLATES PER 19/S1.3 AT 2ND FLOOR U.N.O.
11. (B) INDICATES BEAM DIRECTLY BELOW JOISTS.
    (F) INDICATES BEAM FLUSH W/ JOISTS.
    (H) INDICATES HEADER. PER 15/S1.3 & 17/S1.3 U.N.O.
    (H)* INDICATES WHERE HEADER STRAPPING REQ'D. PER 9&4/S1.1
    LVL INDICATES MICROLLAM
    PSL INDICATES PARALLAM
    LSL INDICATES TIMBERSTRAND
    TB / TB (BLK'G) INDICATES TIE BEAM OF DOUBLE TJI JOIST (OR BLK'G) OR DOUBLE 2x JOIST (OR BLK'G) PER 3/S1.3
12. FOR POSTS, POST TO BEAM CONNECTION SEE 1/S1.3 U.N.O.
13. FOR BEAM TO BEAM CONNECTION SEE 5/S1.3 U.N.O.
14. ALL BEAMS W/ DESIGNATION MARK "P" SHALL BE SEATED AT EA. END ON TOP OF A 4x6 MIN. POST ALL THE WAY TO FOUNDATION TYP. U.N.O. ALL BEAMS W/ DESIGNATION MARK "B" SHALL BE SEATED AT EA. END ON TOP OF A 4x4 MIN. POST ALL THE WAY TO FOUNDATION TYP. U.N.O.
15. SEE SHEETS S4.0 FOR BRACE FRAME, MOMENT FRAME, & WALL ELEVATIONS SHEAR WALL ELEVATIONS.
16. FOR NONBEARING WALL CONNECTION TO FRAMING SEE DET. 4/S1.8
17. STRAP ALL SHEAR WALL OPENINGS PER DET. 2/S1.4 U.N.O. PER ELEVATIONS.
18. REFER TO 18/S1.3 18/S1.3 FOR DIAPHRAGM NAILING SCHEDULE.
19. 6x6/S1.6 INDICATES KING POST SIZE ABOVE HEADER
20. 6x6/S1.6 INDICATES MIN. WOOD POST SIZE ABOVE THIS FLOOR LEVEL WHERE TIEDOWN/HOLDOWN OCCURS THE LARGER SIZE OF POST FROM TIEDOWN/HOLDOWN SCHEDULE SHALL GOVERN.
21. FOR SUPPORT OF MECH. EQUIPMENT REFER TO 4/S1.4 THRU 20/S1.4
22. FOR TYPICAL STEEL CONNECTION DETAILS PER 2/S1.5

FLOOR BEAM SCHEDULE

MARK	SIZE
FP1	3½" x 9½" PSL
FP2	5¼" x 9½" PSL
FP3	7" x 9½" PSL
FP4	5¼" x 12" PSL
B1	6x8 No. 1 D.F.L.
B2	6x10 No. 1 D.F.L.
B3	4x10 No. 1 D.F.L.

JOIST SCHEDULE

MARK	SIZE / SPACING	REMARKS
FJ1	9 1/2 TJI 210 @ 16"	
FJ2	9 1/2 TJI 360 @ 16"	
FJ3	11 7/8" TJI 210 @ 16"	
FJ4	2x10 @ 16" O.C.	
FJ5	2x8 @ 16" O.C.	FLOOR
RJ1	2x6 @ 16" O.C.	ROOF
RJ2	2x8 @ 16" O.C.	ROOF

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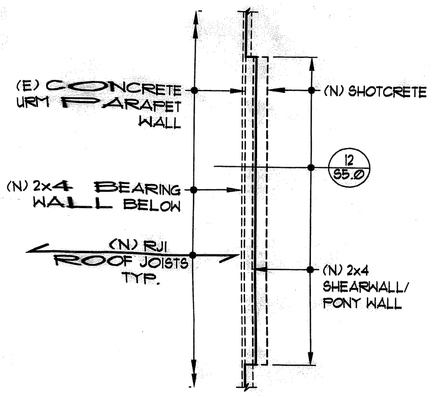
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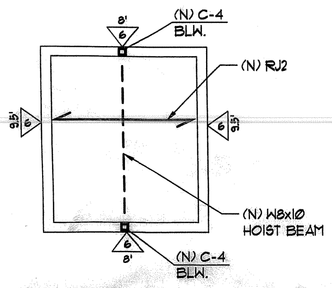
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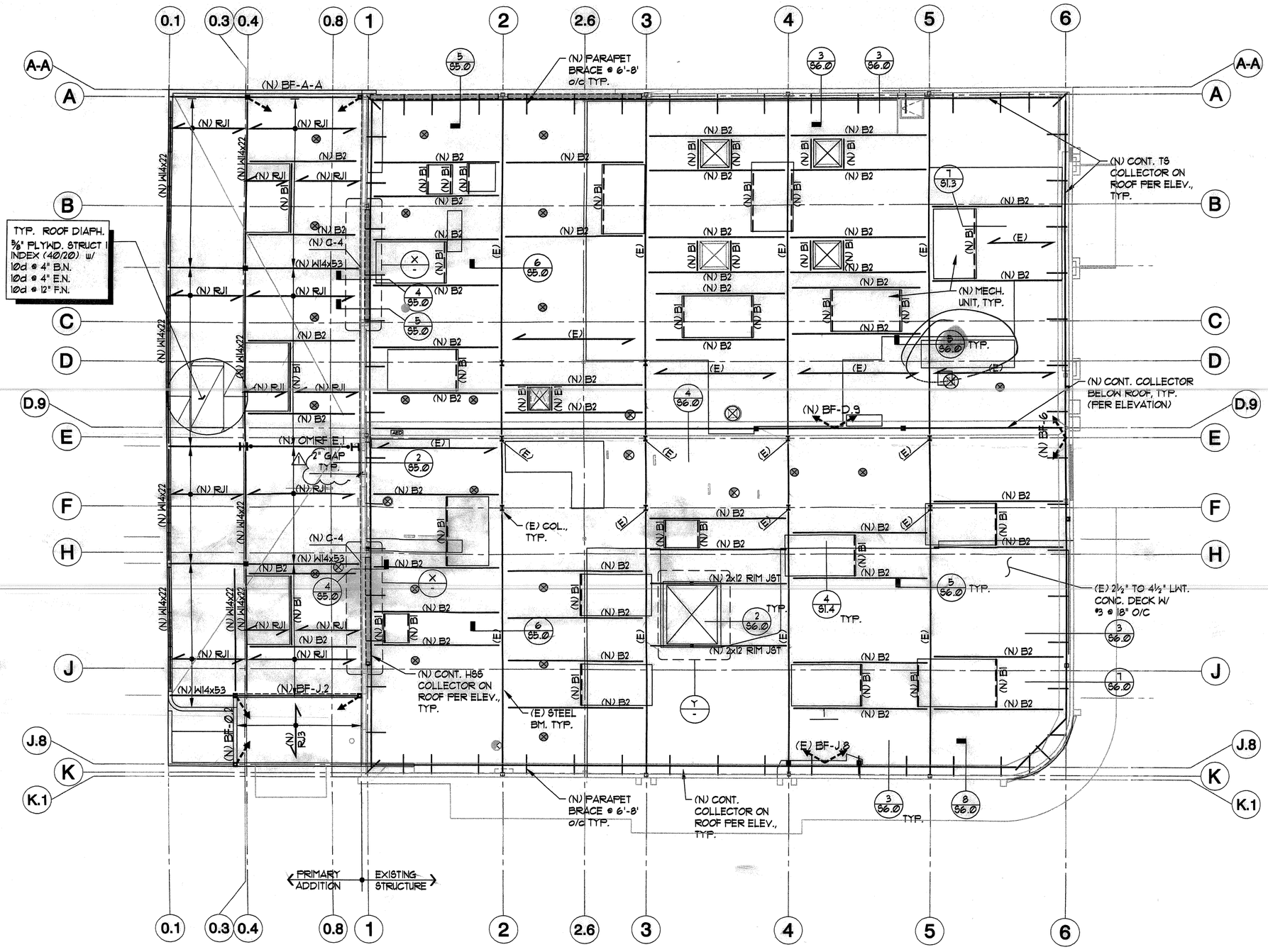
X-PARTIAL ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"



Y-PARTIAL ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"



ROOF FRAMING PLAN

SCALE: 1/8"=1'-0"

ROOF FRAMING NOTES:

1. REFER TO SHEETS S1.Ø TO S1.4 FOR GENERAL NOTES & STANDARD DETAILS. THESE NOTES & DETAILS SHALL BE USED WHERE APPLICABLE WHETHER SPECIFICALLY REFERENCED OR NOT.
2. REFER TO ARCHITECTURAL DRAWINGS FOR THE FOLLOWING:
   A. ALL DIMENSIONS NOT SHOWN.
   B. ALL OPENINGS NOT SHOWN.
   C. ALL NON-BEARING WALL NOT SHOWN.
3. BF-Ø.3 — INDICATES NEW STEEL BRACE FRAME BELOW PER PLAN & ELEVATIONS ON S4.Ø.
   OMRF-E.1 — INDICATES NEW STEEL BRACE FRAME ABOVE PER PLAN & ELEVATIONS ON S4.Ø.
4. INDICATES LIGHT WELL TO BE FRAMED PER 15 S1.3 TYP.
5. (E) INDICATES EXISTING MEMBER
   (N) INDICATES NEW MEMBER
6. INDICATES NEW PARAPET BRACE PER 11 S5.Ø & 12 S5.Ø
7. INDICATES DOUBLE PLATE CONNECTION PER 12 S1.5 TYP.
8. FRAME MECHANICAL UNITS PER S1.4
9. TYPICAL STEEL FRAMING CONNECTIONS PER 2 S1.5 TYP.

ROOF JOIST SCHEDULE

MARK	SIZE / SPACING	REMARKS
RJ1	11 7/8 TJI 21Ø @ 16"O/C	ROOF JOIST
RJ2	2x8 @ 16"	
RJ3	2xIØ @ 12"	

ROOF BEAM SCHEDULE

MARK	SIZE	REMARKS
F1	3½" x 9½" PSL	
F2	5¼" x 9½" PSL	
F3	7" x 9½" PSL	
B1	4x12 No. 1 D.F.L.	WHERE SHOWN DASHED ON PLAN, INSTALL AS SOLID BLK'G. BTWN. (E) JOISTS
B2	6x12 No. 1 D.F.L.	
B3	4xIØ No. 1 D.F.L.	

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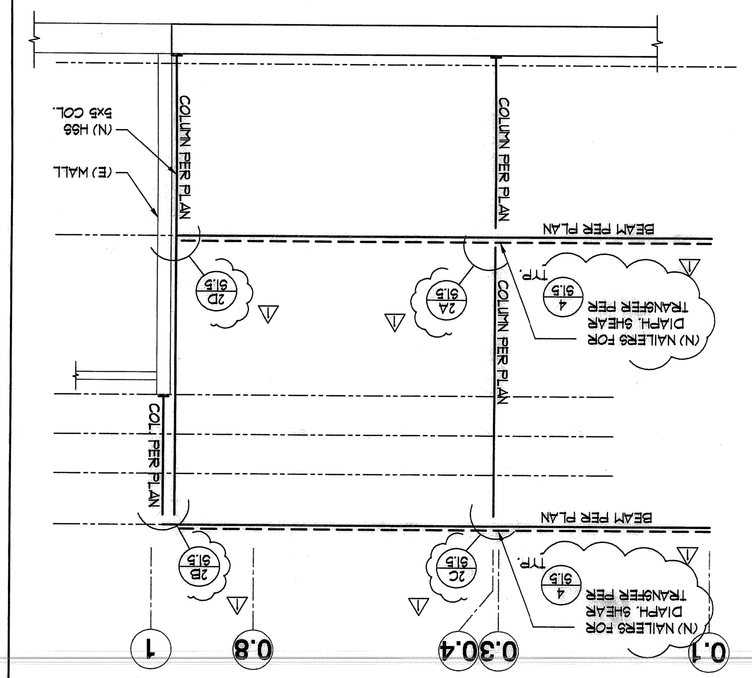
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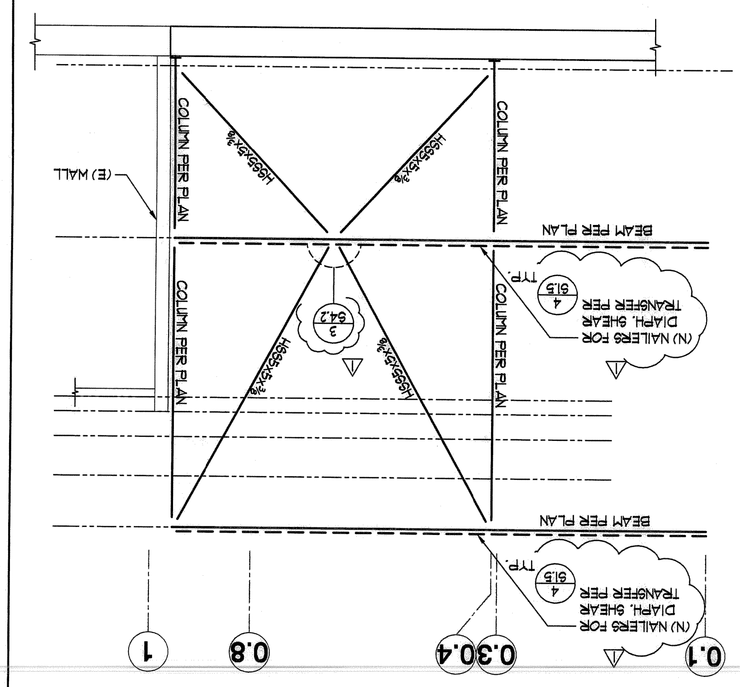
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S3.2



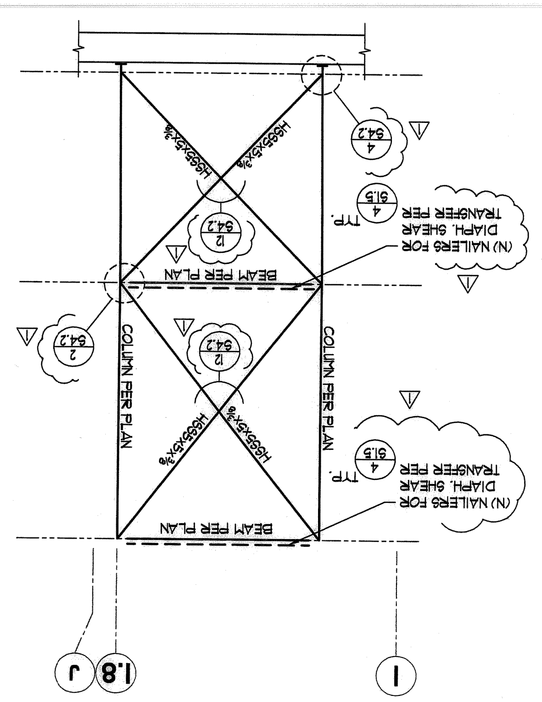
SECTION ALONG GRID LINE B.5

SCALE: 1/4"=1'-0" — B.5



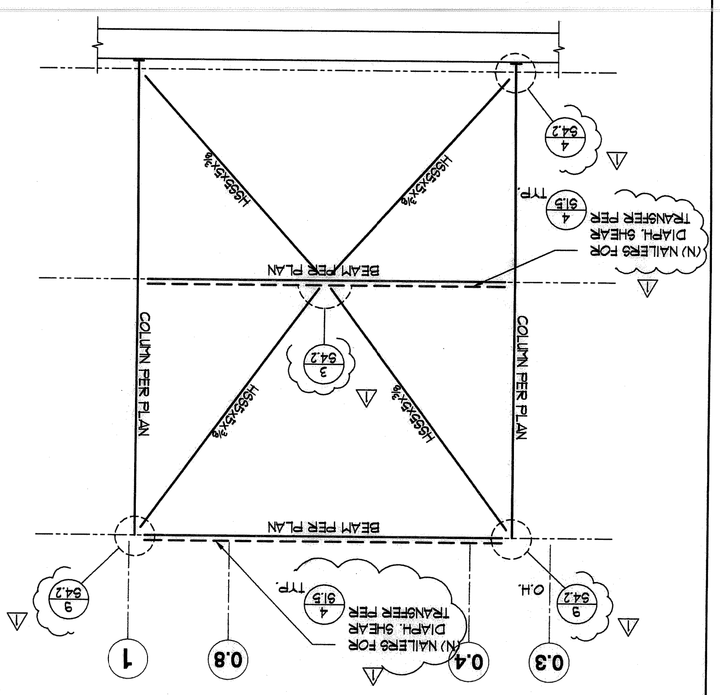
BRACE FRAME ALONG GRID LINE A-A

SCALE: 1/4"=1'-0" — BF-AA



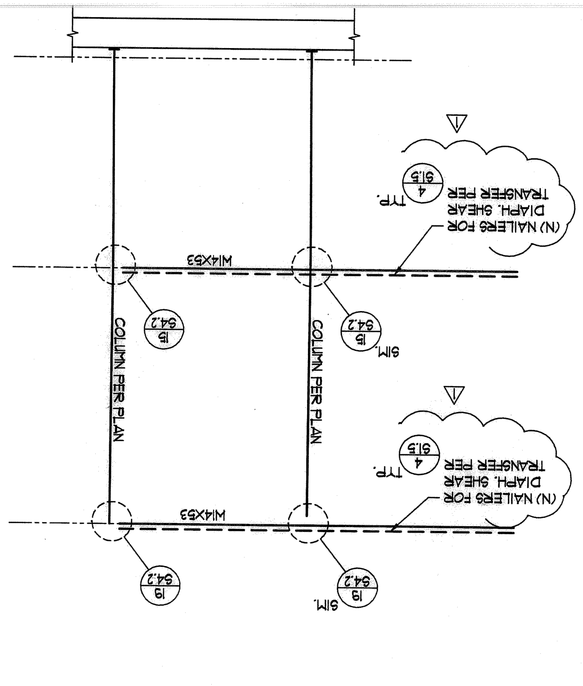
BRACE FRAME ALONG GRID LINE 0.2

SCALE: 1/4"=1'-0" — BF-02



BRACE FRAME ALONG GRID LINE J.2

SCALE: 1/4"=1'-0" — BF-J.2



MOMENT FRAME ALONG GRID LINE E.1

SCALE: 1/4"=1'-0" — OMRF E.1

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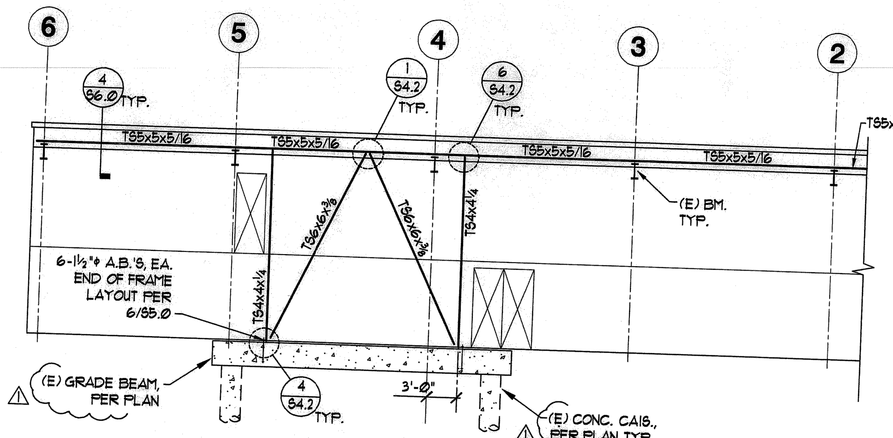
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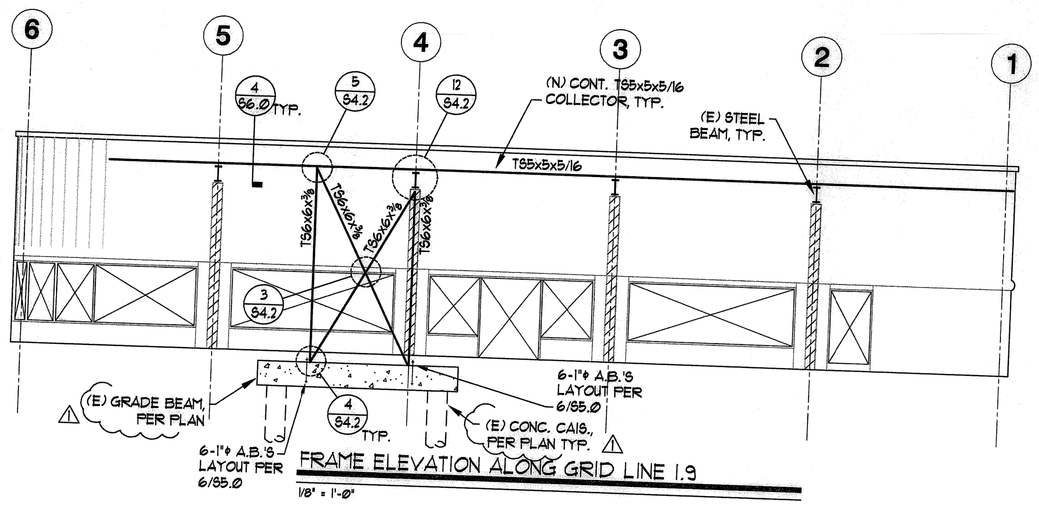
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SA.0



FRAME ELEVATION ALONG GRID LINE D.9

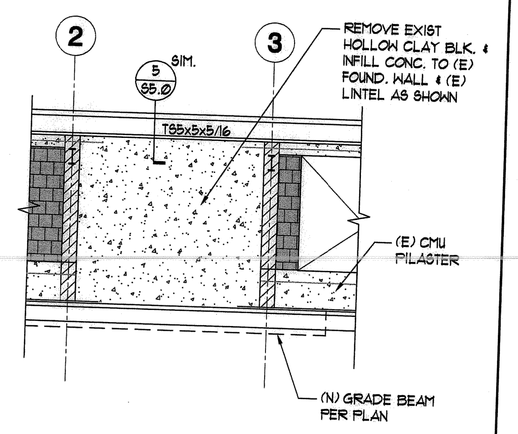
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FRAME ELEVATION ALONG GRID LINE 1.9

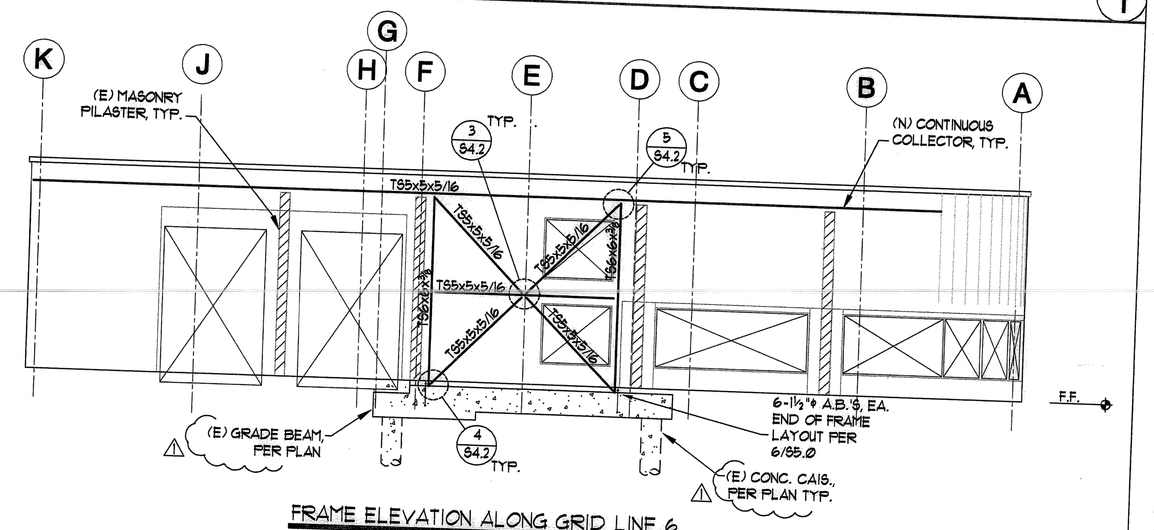
1/8" = 1'-0"

1



TYP. SHOTCRETE BAY INFILL

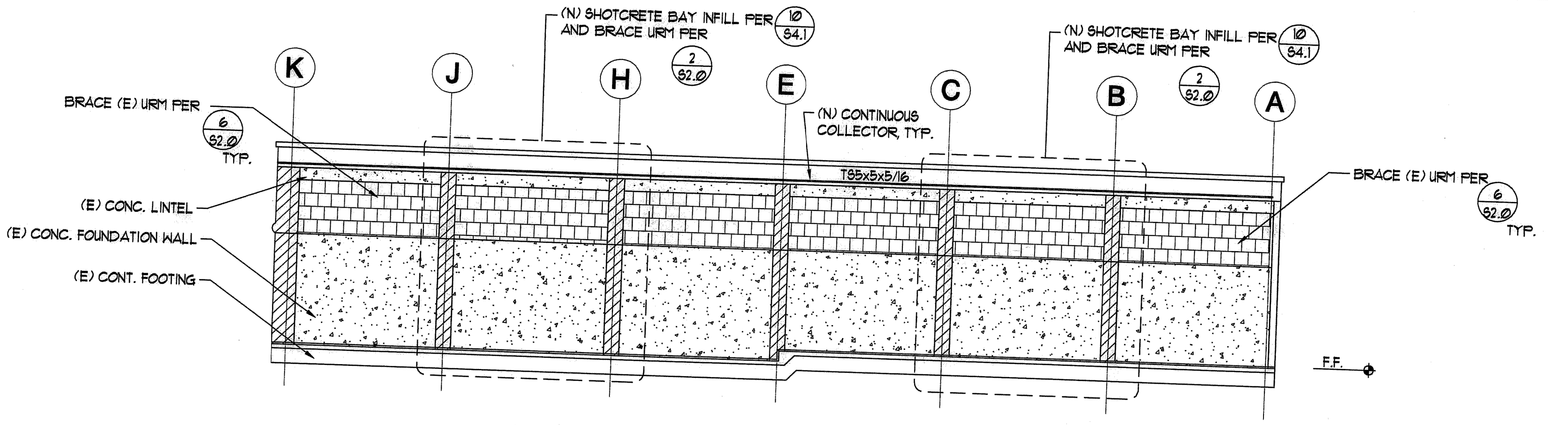
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FRAME ELEVATION ALONG GRID LINE 6

1/8" = 1'-0"

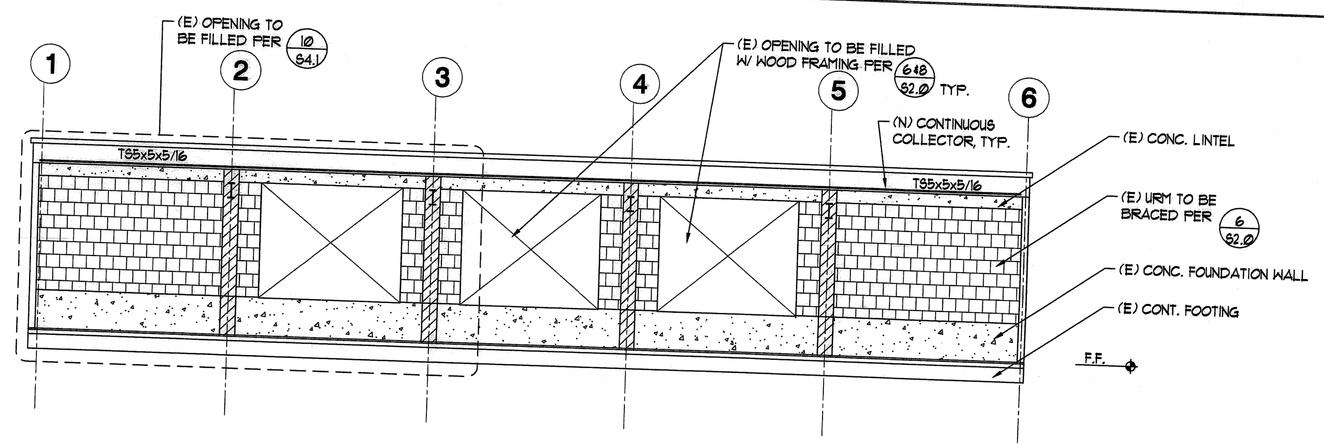
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INTERIOR ELEVATION ALONG GRID LINE 1

1/8" = 1'-0"

3



INTERIOR ELEVATION ALONG GRID LINE A

1/8" = 1'-0"

4

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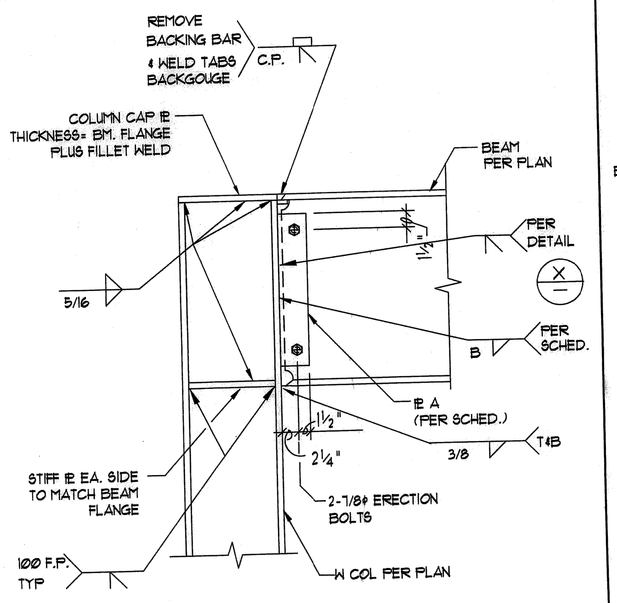
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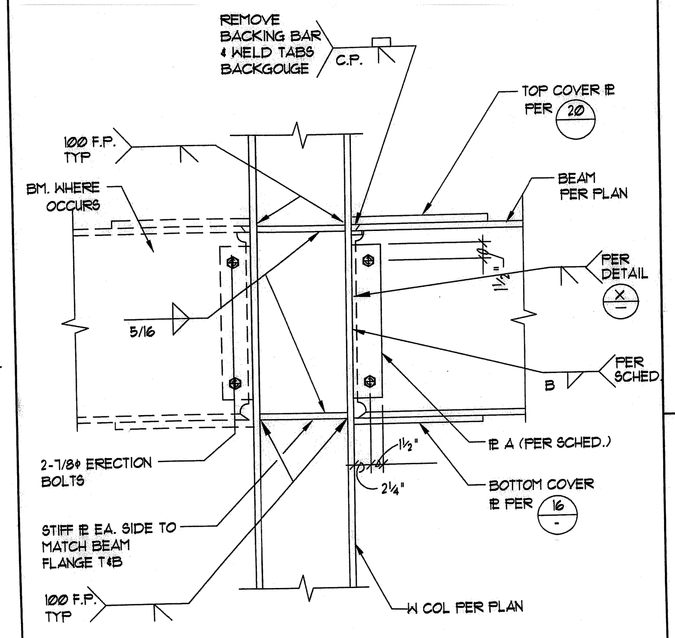
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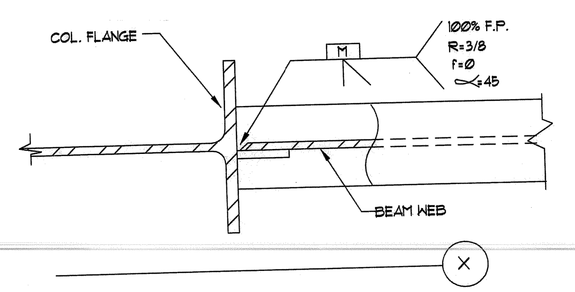


TYP MOMENT CONN. TO COLUMN FLANGE AT ROOF



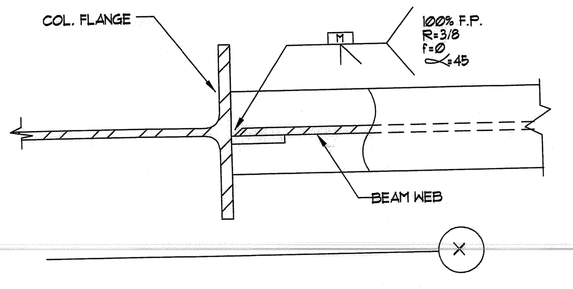
TYP MOMENT CONNECTION TO COLUMN FLANGE AT FLOOR

NOTE: COMPLETE PENETRATION WELD SHALL BE MADE USING WELD METAL W/ CHARPY V-NOTCH TOUGHNESS 20 FT-LBS AT Ø F

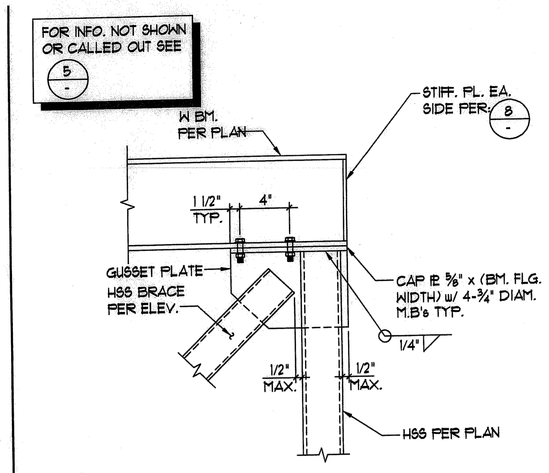


TYPICAL MOMENT CONNECTIONS SCHED.			
BEAM SIZE	PLATE "A" "THICKNESS"	WELD "B"	REMARKS
W12 X	3/8"	1/4"	ROOF
W14X	3/8"	1/4"	ROOF

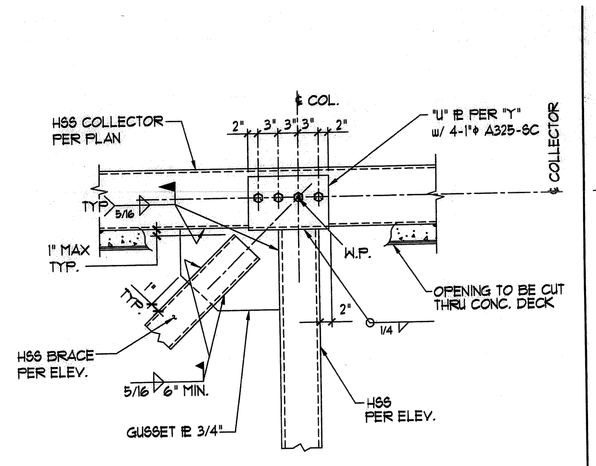
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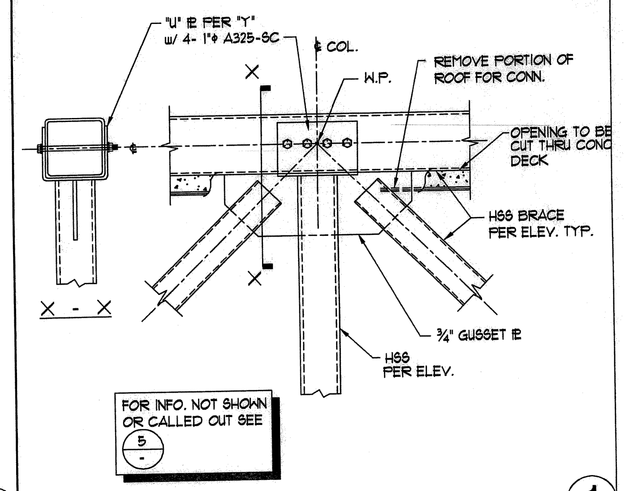
TYPICAL MOMENT CONNECTIONS SCHED.			
BEAM SIZE	PLATE "A" "THICKNESS"	WELD "B"	REMARKS
W12 X	3/8"	1/4"	FLOOR
W14X	3/8"	1/4"	FLOOR



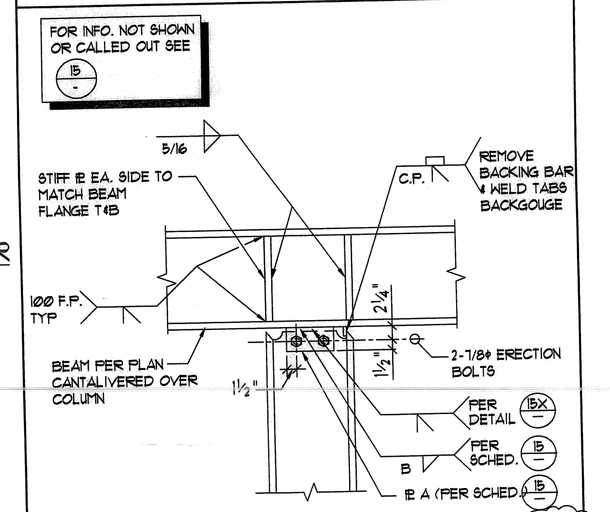
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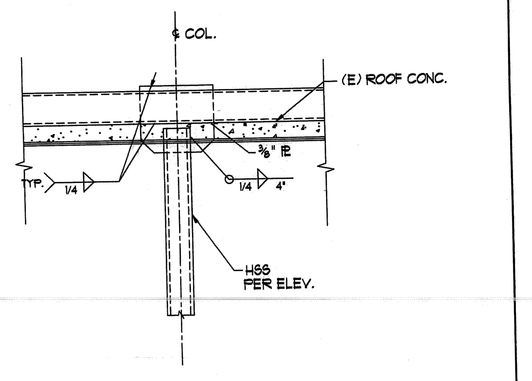
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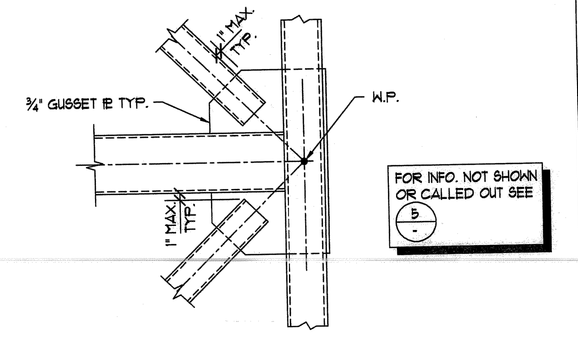
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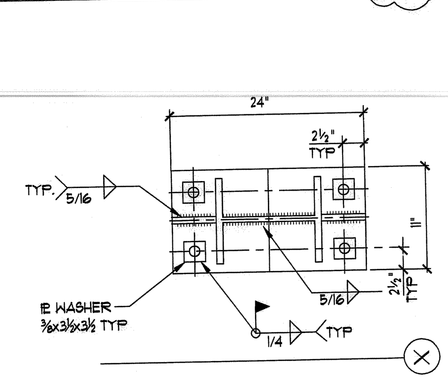
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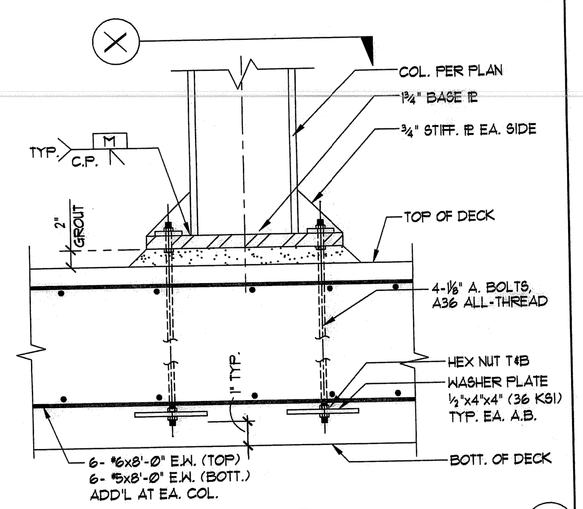


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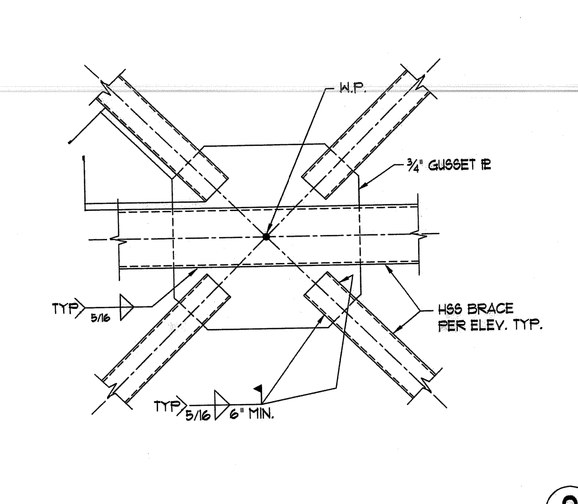
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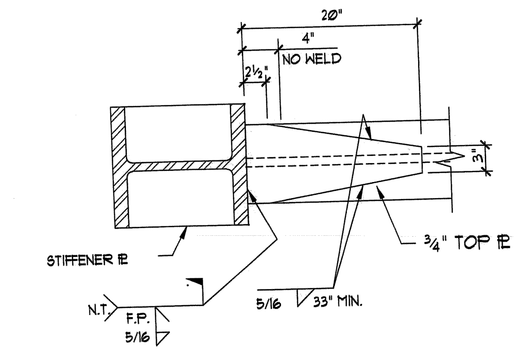


TYP MOMENT FRAME CONNECTION TO BASE

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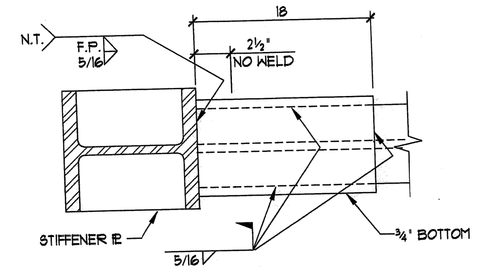


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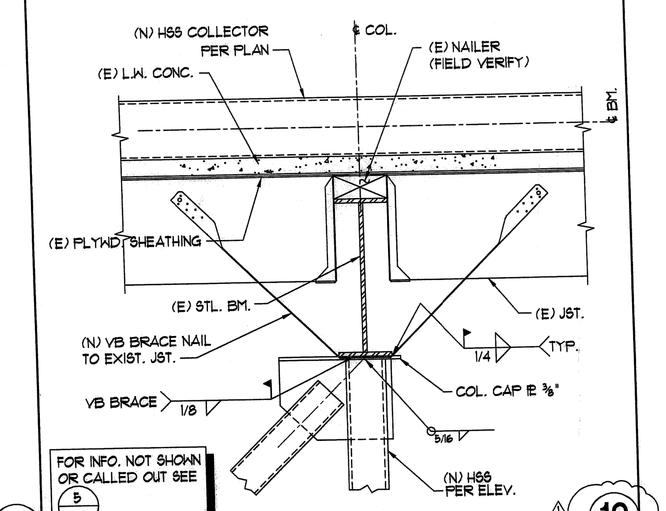
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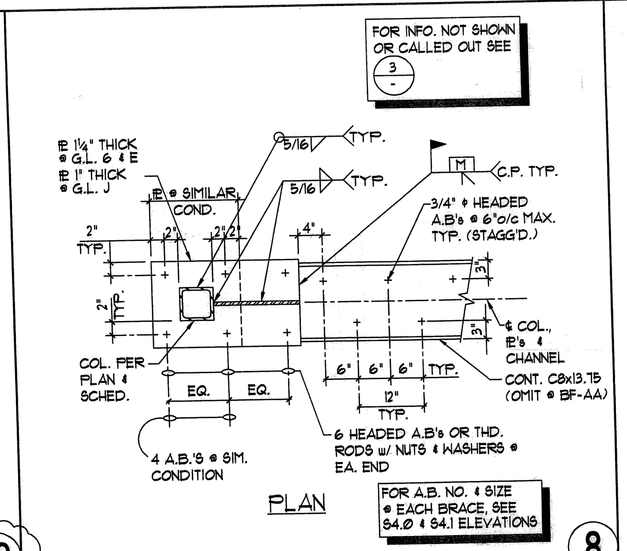
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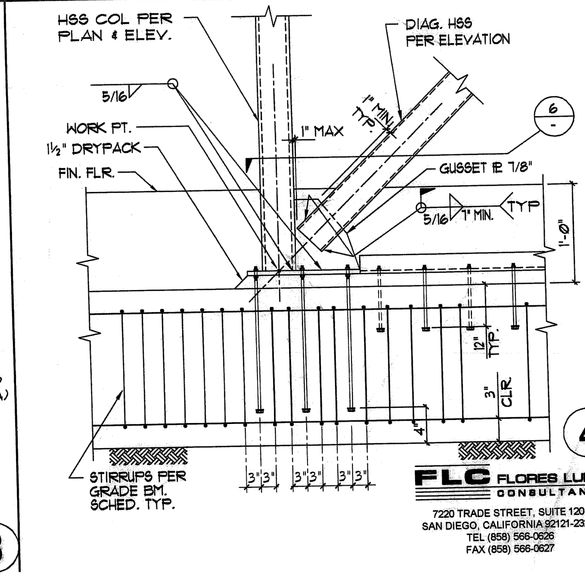


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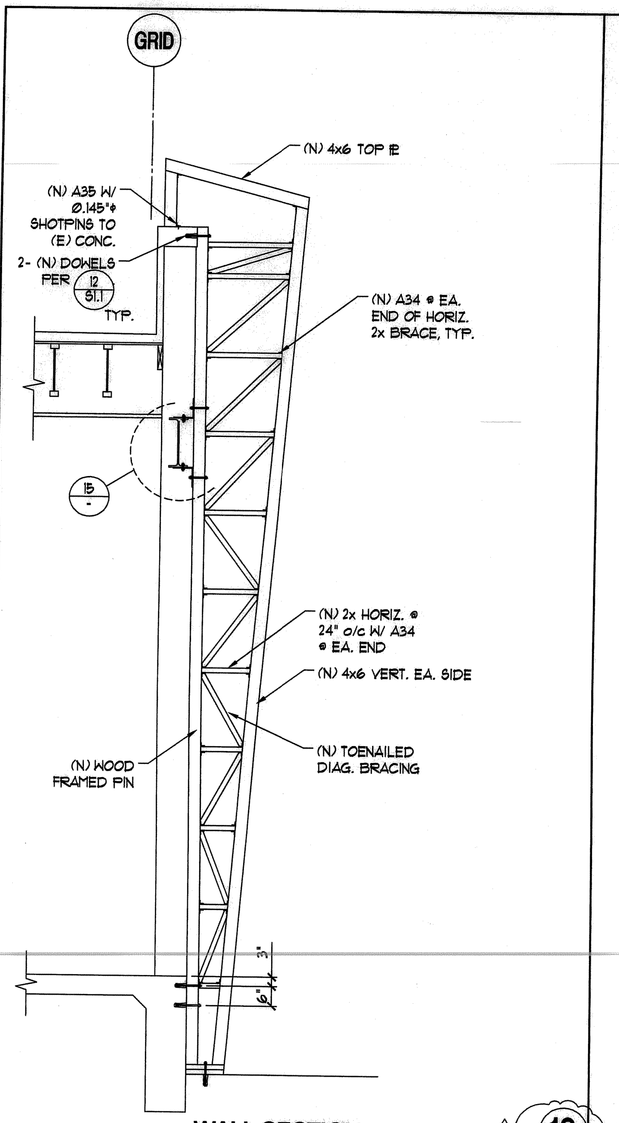
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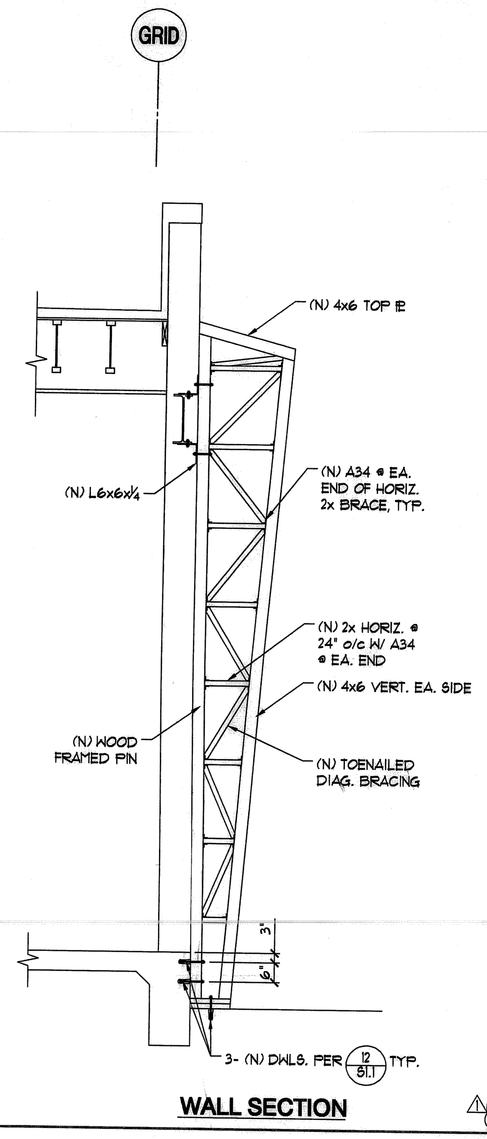
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Issue Dates	
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Design Development	
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Drawn By	
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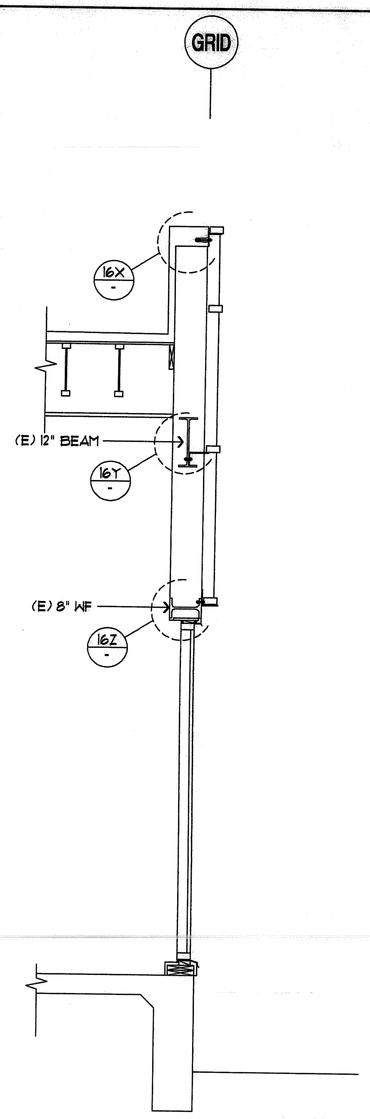
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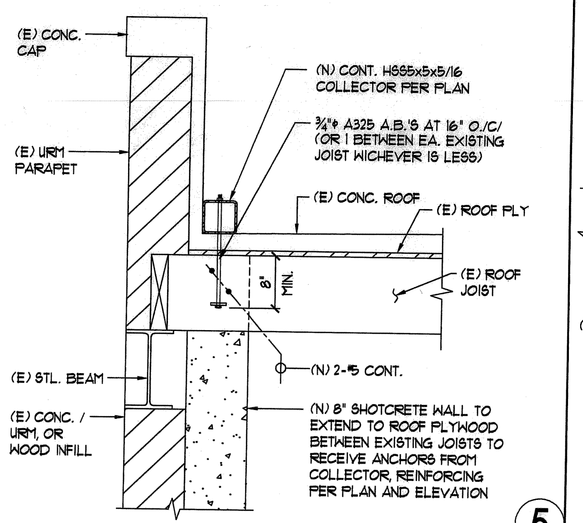
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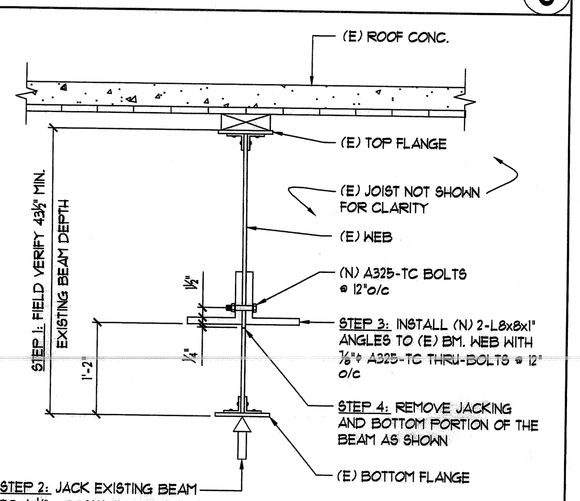
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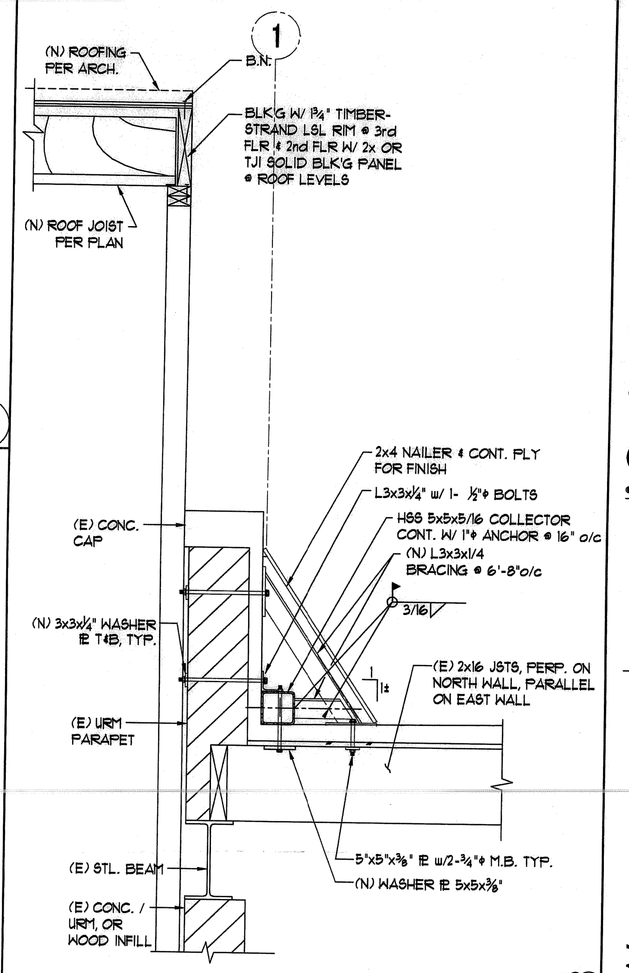
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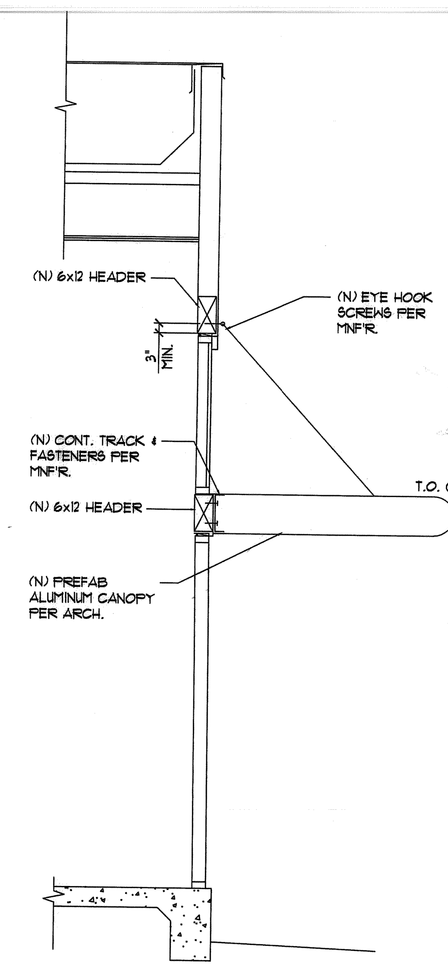
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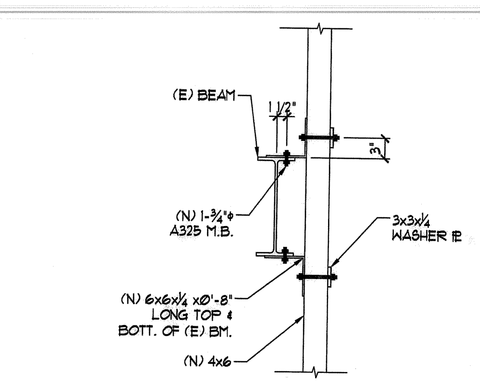
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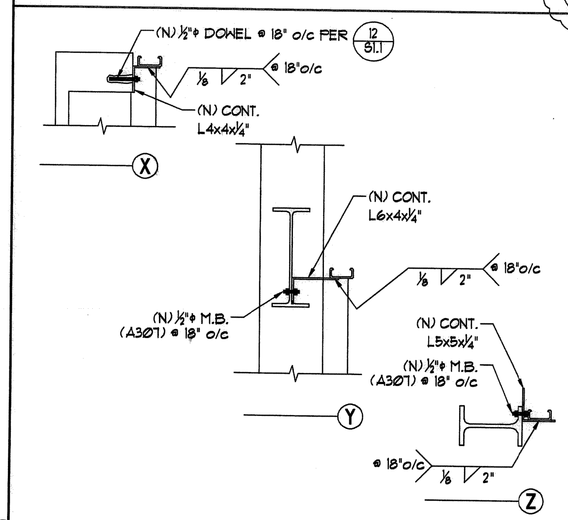
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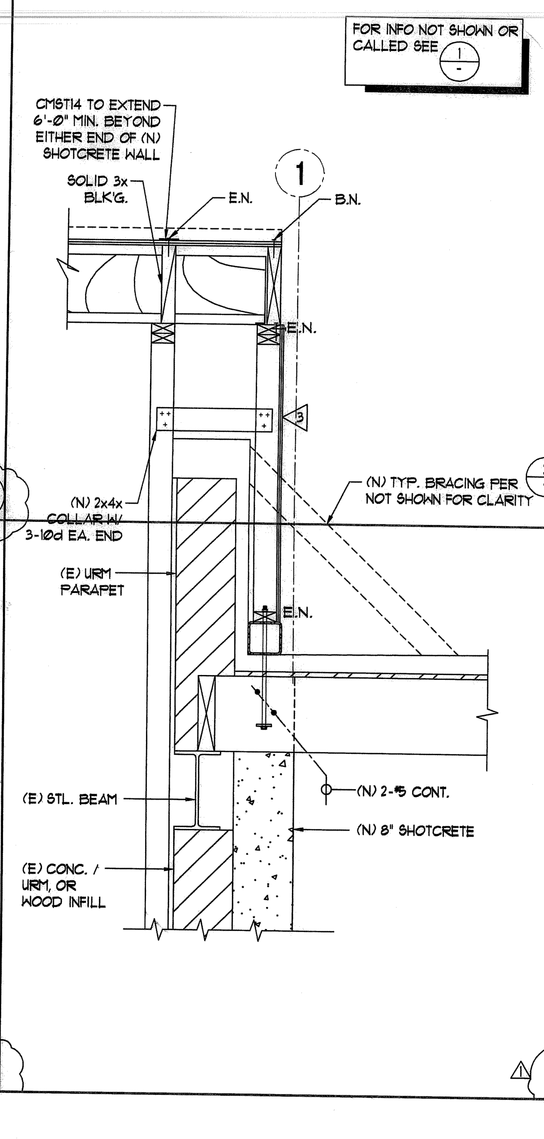
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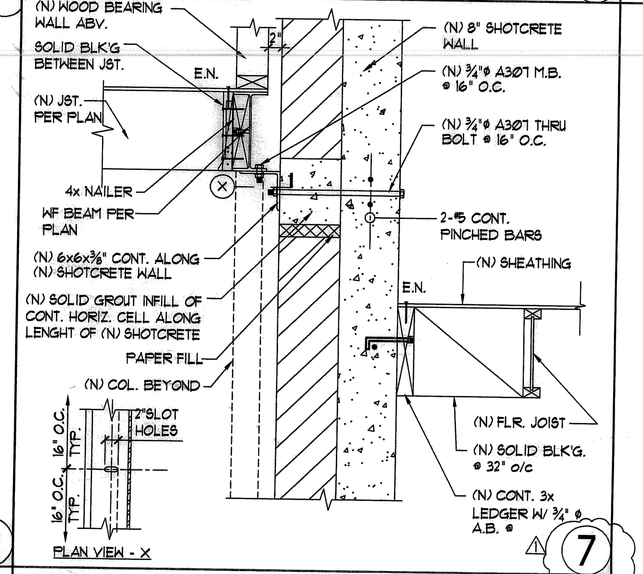
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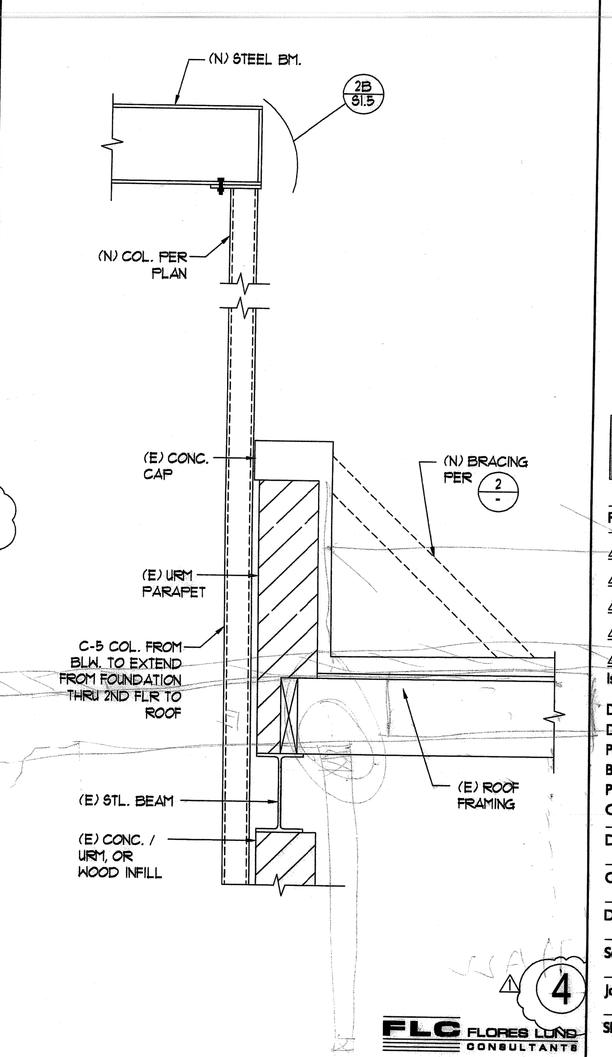
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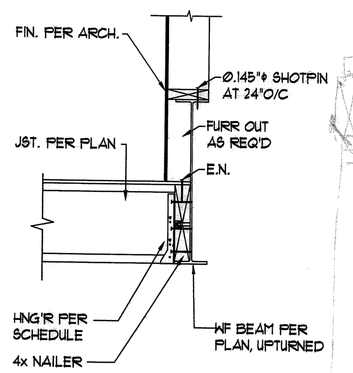
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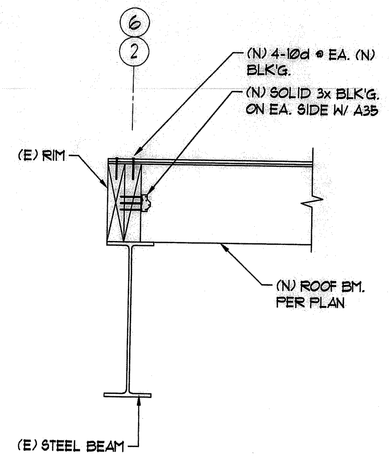
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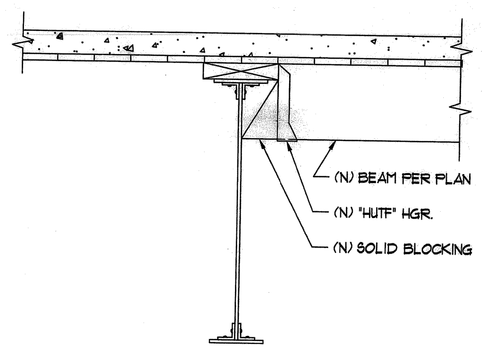
Senior Community Centers

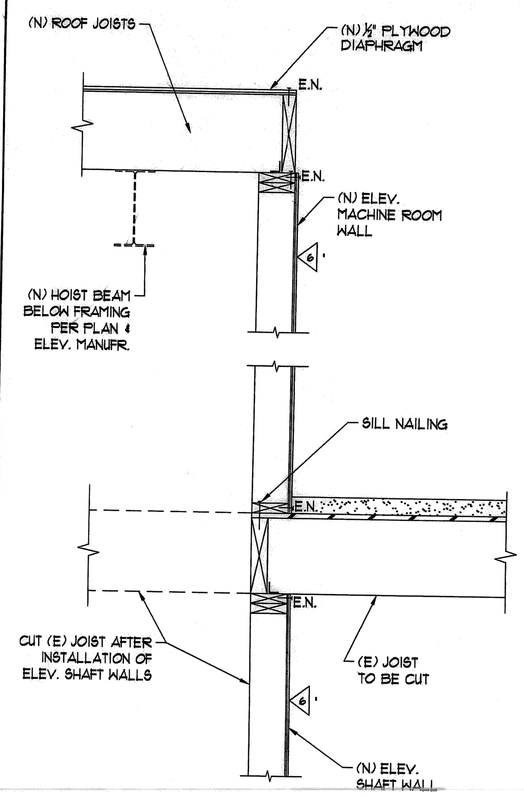
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Design Development	.
Plan Check	6.15.09
Bid Set	.
Permit Set	.
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Check By	RHF
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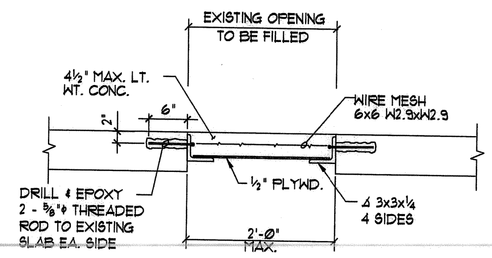
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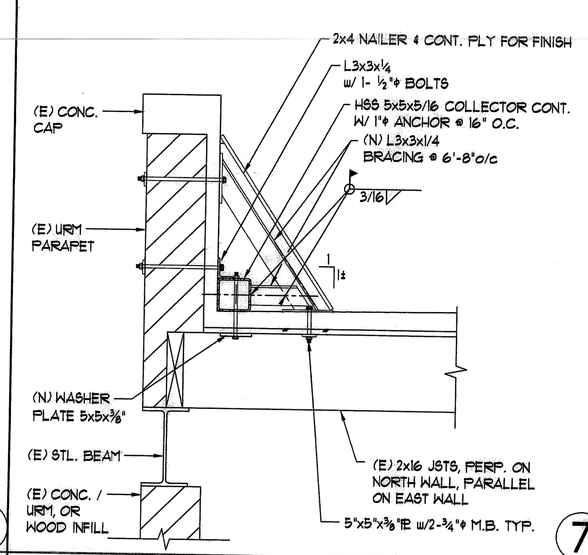


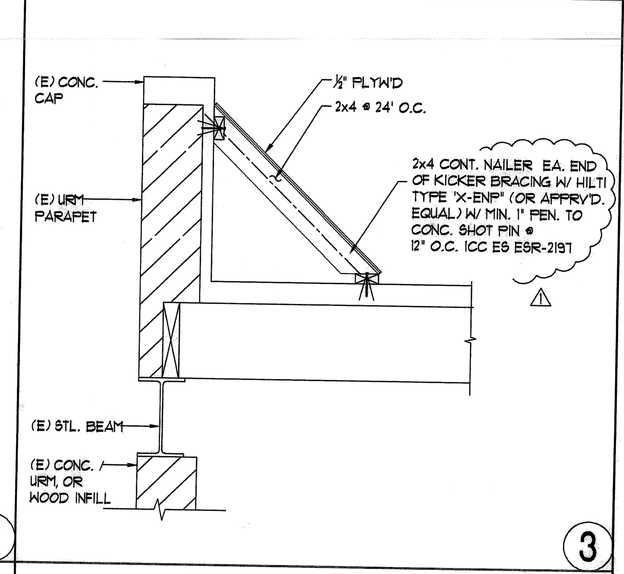


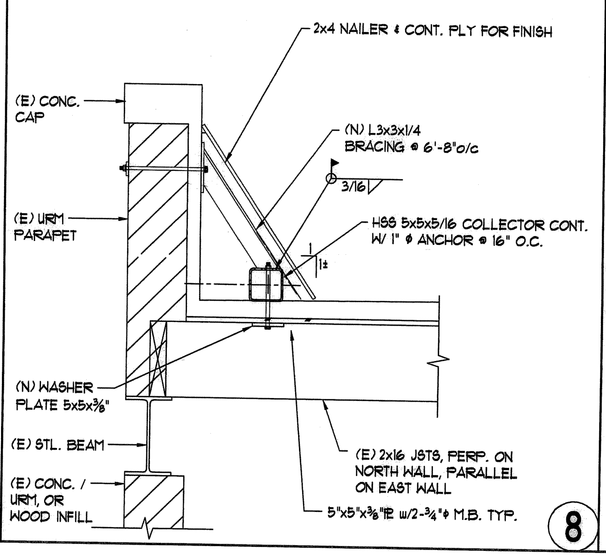


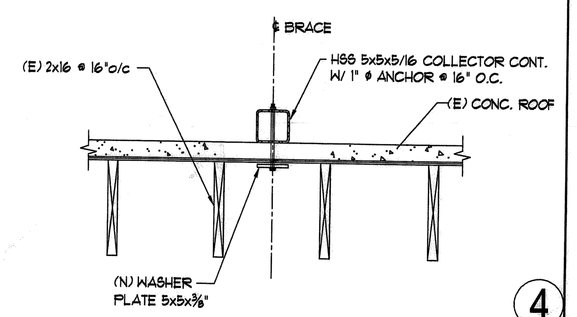


EXIST. ROOF DECK OPN'G. TO BE FILLED









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